Stormwater
Design Guidelines

City of Cibolo

Stormwater Design Guidelines

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Introduction

SECTION I

INTRODUCTION

A. Purpose

The standards and criteria in this document are promulgated to implement the intent of the stormwater management ordinances adopted respectively by the City of Cibolo for use in their respective jurisdictions. The term **"Guidelines"** is used throughout this document in reference to itself. The objective is to encourage uniformity of results through the use of unified criteria and sound practices in the planning, analysis, design, and construction of drainage facilities.

B. Source of Authority

These Guidelines are regulatory in nature, deriving their authority from the stormwater management ordinances and floodplain management ordinances adopted from time to time by the City Council of the City of Cibolo.

C. Definitions

Unless specifically defined in these Guidelines and/or in the Glossary, Appendix F, words or phrases used in these Guidelines shall be interpreted so as to give them the meaning they have in common usage and to give these Guidelines their most reasonable application. Responsibility for final interpretation of the meaning of language used herein rests with the City Engineer of the City of Cibolo.

D. Considerations

Managed Stormflow

One of the basic purposes of these stormwater Guidelines is to assure that newly developing land areas are planned and designed in a manner that safeguards life, property, and public infrastructure from damage due to ill-managed storm flow.

Guidelines Apply

Inasmuch as platting must provide for right of way and easements that assure efficient conveyance of storm flow within streets, storm drains, and prepared swales or channels, these guidelines are applicable to all such platting proposals. Likewise platting must demonstrate suitable spatial relationships between proposed building sites and floodplain areas designated by the Federal Emergency Management Administration (FEMA). For these reasons, anyone interested in building real property or public or service infrastructure of any kind in Cibolo is obligated to demonstrate to the City that they are in substantial compliance with these Guidelines. Such compliance will be one of the measures by which the adequacy of any proposed land plan, preliminary plat, final plat, or site plan will be evaluated.

<u>Section II</u>
Policies

SECTION II

POLICIES

A. Stormwater Principles

Drainage System For purposes of regulation, the drainage system shall be divided into geographical and functional groupings. The drainage system consists of all natural and man-made features that collect or receive concentrated stormwater flow. Examples are swales or channels (natural or man-made), streets, storm sewers, minor streams and major streams.

Primary and Secondary Functional division is separation of the drainage system into its primary and secondary components. The Primary System consists of major streams that convey collected stormwater through and out of the City, including primary tributaries thereof. The Primary System is made up of the watercourses that are part of the FEMA-designated floodplain management network, the geographic limits of which may be amended from time to time by the City. The Secondary System consists of all minor drainage ways, streets, storm sewers, and swales that collect stormwater and convey it to the Primary System.

Storm Duration From a hydrologic standpoint, the Secondary System is sensitive to short duration, high intensity rainfall events. Flood effects occur suddenly and dissipate quickly, usually within a period of a few hours. By contrast the Primary System is sensitive to longer duration, moderate intensity rainfall events. Flood events occur over a longer period, with a slower rise to the fall from peak flows and flood elevations. This fundamental difference between the Primary and Secondary Systems forms the basis for strategies to manage stormwater and its effects within each.

Unique Characteristics Geographical division involves separating the various streams and land areas into broad drainage areas having unique characteristics in terms of land cover, pattern of development, governmental jurisdiction, proposed land uses, and system interconnection. Recognition of these differences allows for logical formulation of policies and standards tailored to specifics rather than generalities.

Known Problems Because the basic reason for regulating stormwater runoff and conveyance is to promote public safety, it must be emphasized that where persistent, known drainage problems exist, criteria more stringent than stated in these Guidelines may be necessary.

B. Framework of Stormwater Management Terms

A great variety of terms are used in the science and administration of managing urban stormwater. To foster clarity and expediency in use of these Guidelines, a limited series of terms has been specially defined. The focus is on the definitions of drainage areas, land proposed for development, and the purposes of detention. The diagram in Figure II-1 offers a graphical representation supporting this framework of terms. The principal terms coined below are in bold print in this Section and are capitalized throughout these Guidelines. The Glossary in Appendix F provides specific definitions of these and other key terms.

1. Watersheds

Every land area in the Cibolo region is in a "**watershed**" of some description, which is associated with some kind of watercourse. For managing storm runoff in these areas it is useful to divide these areas according to the watercourses that drain them.

Named Streams

For purposes of these Guidelines "**watersheds"** are all of the land areas contributing storm runoff to each of the principal watercourses making up the primary system. The primary system is divided into logical parts that are referred to as the "**Named Regulatory Watercourses"** listed in Appendix B.

A hypothetical "Principal Named Watercourse" and the hypothetical watershed ("Watershed A") it drains are sketched in Figure II-1.

2. Basins

Tributaries

For purposes of these Guidelines a "**basin**" is defined as the land area drained by a tributary of a "Principle Named Watercourse". Each "Principal Named Watercourse" has several tributaries (some possibly having localized names) that serve to help drain the **watershed**. Each **watershed** is made up of several **basins,** and all areas in a **watershed** are considered to be part of one of its **basins.**

Specific Limits

The specific geographic limits of any **basin** are a function of topographic features that can only be determined through engineering study. Such limits must be determined when dictated by the characteristics of a proposed land development project as determined by the City Engineer or his/her designee during project review processes.

Figure II-1 illustrates the **basins** of a hypothetical **watershed**. In this sketch the "Principal Named Watercourse" has six tributaries, so the **watershed** is considered to have six **basins**. **Watershed** "A" has six identified **basins,** basins 1, 2, 3, 4, 5 and 6.

3. Land Development Projects

a. Land Areas

Enhanced Consistency Land development projects occur in many shapes and sizes in a variety of locations. These Guidelines apply to all proposed projects but their application is a function of numerous variables. To enhance consistency in determining how these Guidelines apply to particular situations, the following land area terms will be used.

Project Area **Project Area**: The entire land holding associated with any proposed land development project will be considered the **"Project Area"**. This is to include the largest acreage of any combination of: the entire ownership, the entire parent tract, and/or the entire purchase option acreage, if any. This is true for all contiguously owned tract(s) or lots regardless of whether platted or not platted. It is also irrespective of whether construction (buildings or infrastructure) is planned on portions of the land near term and/or at some future time, however well or poorly defined.

2-Phase Project In Figure II-1 hypothetical Project B is a two-phase project. Stormwater analysis and design for Phase 1 of Project B must consider Phase 2 to be part of the **project area**, even if Phase 2 facilities and/or buildings are planned for future construction. In addition, it must consider any "**Above-Project Area(s)"** and "**Pathway Area(s)**" as described below.

Above-Project Areas **Above-Project Areas**: These are any land areas that contribute storm runoff onto or through the **project area**. In Figure II-1 schematic projects A, C, and E all have **"above-project areas"** since upland areas contribute storm runoff to the **project areas**. Schematic projects "B" and "F" may or may not receive runoff from limited upland areas. Schematic Project "D", in Basin 1, boarders the **basin** divide and receives no runoff from upland areas, so it has no **above-project area**.

Pathway Areas **Pathway Areas:** As described in Paragraph C2 of this Section, "designated conveyance pathways", however simple or complex, must be identified for every land development project. Conveyance pathways downstream of a **project area** may carry runoff from land that is not part of the **project area** or the **above-project area**. Areas discharging to a "conveyance pathway" downstream of the **project area** are considered "**Pathway Areas"**.

Two Basins In Figure II-1 Projects "A", "B", and "D" each include **pathway areas** along the "conveyance pathway" that would extend from the **project area** to the tributary, then to **Watercourse A**. Project "F" straddles the divide between basins, so it will have two "conveyance pathways" and two sets of **pathway areas**, one in each of the two **basins**. The extent of analysis, design, and improvement for the conveyance pathway and the land areas it drains varies as stipulated elsewhere in these Guidelines.

Drainage Study Area

Drainage Study Area: Every project will be considered as having a **"Drainage Study Area"** that is the **project area** at a minimum. As applicable, it may also include **above-project area(s)**, and/or **pathway area(s)**. To be considered complete, a "drainage study" must address all three components of a **drainage study area,** as well as the conveyance pathway itself to limits as determined under provisions of Paragraph D2 of this section. If such areas do not exist for a particular project, it shall be so stated in the drainage study report.

Design Drainage Area

Design Drainage Area: Every **drainage study area** will include any number of **"Design Drainage Areas"** that must be analyzed to determine the design storm flow for the purpose of sizing and placing stormwater management facilities of various types. This can vary widely, from a small area draining to a curb inlet, to many acres served by a channel and culvert.

b. Purposes of Detention

Two Purposes

Detention is a useful stormwater management technique. As fully addressed in Paragraph C3 of this Section, it can be used for managing flood control over a broad area such as an entire **basin** or **watershed**. It can also be used to manage property-to-property conveyance of stormwater. Whether detention is required by these Guidelines is partially a function of how a **project area** is situated in a **watershed**. This gives rise to three types of detention as a function of the purpose.

Not Design Type

"Type" in this context does not relate to design characteristics of facilities used to accomplish detention objectives.

Flood Control

Type 1 Detention (Flood Control): The purpose of this type of detention is to manage runoff from a **watershed** or **basin**. A **project area** located near the bottom of a **watershed** will generally not require detention for this purpose. Schematic Project "E" in Figure II-1 illustrates this condition.

Conveyance Mgmt.

Type 2 Detention (Conveyance Management): The purpose of this type of detention is to manage the delivery of runoff from a property to neighboring (generally adjoining) properties. This may be necessary regardless of how a **project area** is situated along the length of a principal watercourse. In Figure II-1 schematic project "D" illustrates this condition because it may be low enough in the **watershed** not to warrant **Type 1 Detention**.

Dual Purpose

Type 3 Detention (Dual Purpose): Detention in this category is considered to have a dual purpose. It is important for both flood control and managing property-to-property conveyance. Schematic projects "A", "B", and "F" illustrate this condition. All three projects must drain to or through adjoining properties to reach a tributary, so detention may be required to satisfy conveyance criteria. In addition, because they are situated in the upper areas of a **watershed**, managing the peak discharge from them is likely to contribute to flood control objectives for the **watershed** as a whole or for the **basin** in which each is located.

No Detention

In Figure II-1 schematic "Project C" illustrates a situation where detention may not be warranted. If low enough in the **watershed**, **Type 1 Detention** may be unnecessary, possibly even detrimental, to flood control objectives. Moreover, because it can drain directly into the principal watercourse, there may be no need for **Type 2 Detention**.

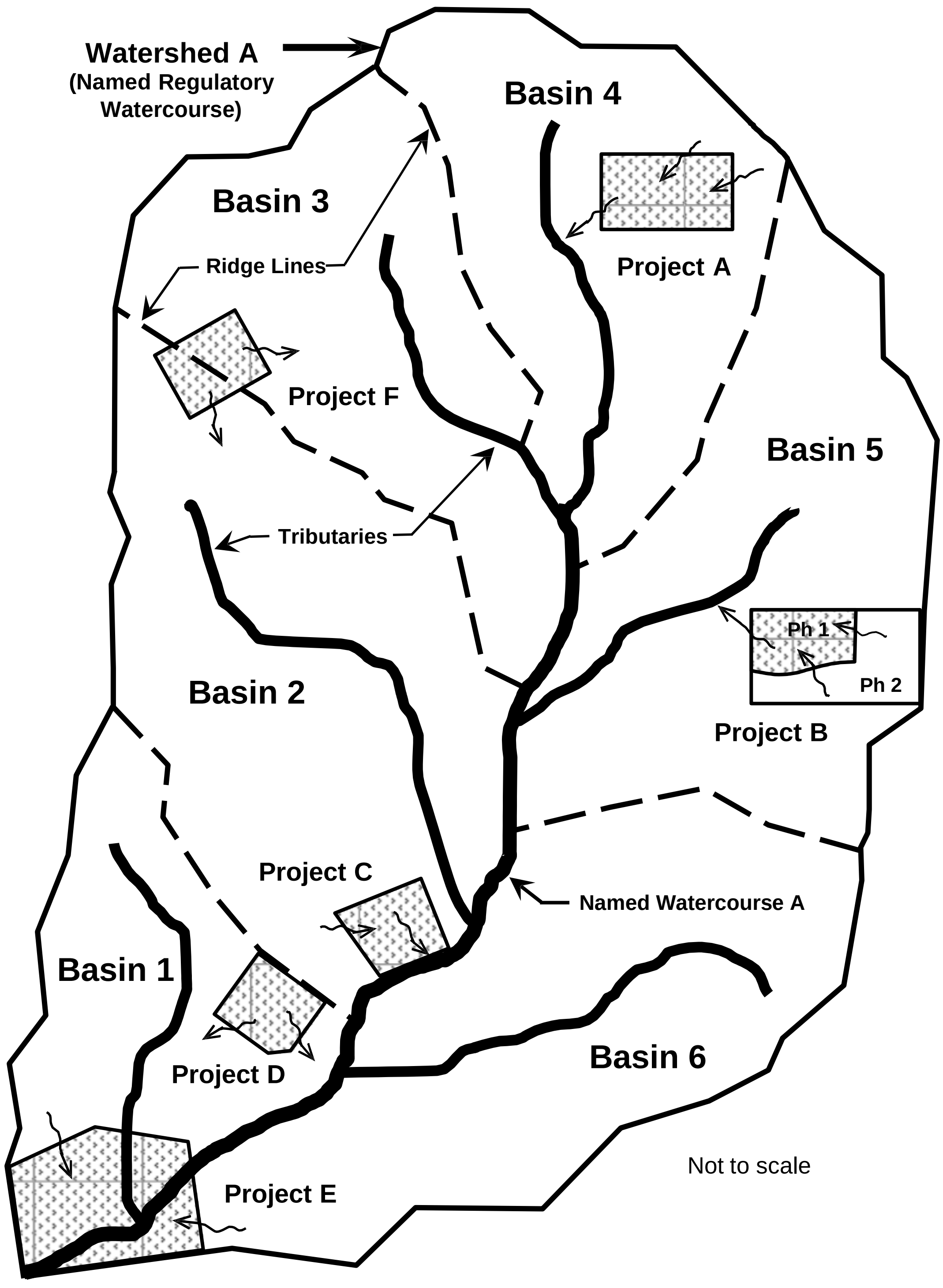


Figure II-1: Hypothetical Watershed – Basin – Projects Diagram

C. Watershed Management

1. Primary Drainage System

a. Nature of Problems in Primary System

Floodplains

Stormwater problems in the primary drainage system result from floodwaters rising out of the banks of natural streams and inundating adjacent natural floodplains. Symptomatic problems are flooding of building structures, overflow of bridges and culverts hampering traffic access, and damage to public & private infrastructure.

Problem Causes

Problems in the primary system can be caused by the following:

- Inadequate capacity of crossing structures and failure to allow for overflow.
- Placing the finish elevation of the lowest floor of a structure situated adjacent to the Primary System below the existing or ultimate 100 year flood elevation.
- Inadequate or out-dated engineering studies that form the basis of the regulatory flood elevations.
- Failure to allow for increased discharge from, and resulting flood elevations in, upstream areas.
- Failure to control & limit increased stormwater discharge to downstream areas.
- Improper or ineffective alterations to natural channels that have the effect of "transferring" flood problems to upstream or downstream areas.

Resulting Hazards

The results are creation of hazards to life and damage to public and private properties. Remedial measures usually involve large capital improvements to channelize streams, create large detention facilities, or build larger crossing structures for roadways.

Hydrologic Studies

As a first step to dealing with these problems, the City adopted comprehensive hydrologic and hydraulic engineering studies for most of the primary system and tributaries thereof. These identify the flood discharge and flood elevations within the primary system, for existing and ultimate development conditions. Ultimate development conditions reflect the drainage situation as expected if the development within the City follows that projected in the City's adopted comprehensive land use plan. In theory, the existing and ultimate flood conditions are known. Duly adopted flood studies will govern actions and treatments (whether public projects or associated with land development projects) that affect the primary system and its tributaries, consistent with state and federal regulatory requirements.

Minimize Flooding

The policy of the City is to encourage the efficient conveyance of stormwater through and out of the City within the primary system. The lowest floor of all structures adjacent to the primary system shall be kept at a level above the ultimate 100-year flood level, and no structure will be allowed within the existing 100-year flood path defined as the "floodway." In order to eliminate sporadic and uncoordinated site improvements, modification of the floodway shall be restricted to projects engineered and treated in conformance with a comprehensive master plan established for regulatory channel reaches.

Encroachments

Unless stipulated otherwise in a City ordinance or other design guidelines, minor encroachments in the floodway fringe will be allowed for individual sites and developments, provided they are clearly part of a "Drainage Development Permit" approved by the City. Crossing roadway structures are allowable to include encroachments, provided they are designed to accommodate the range of ultimate design flows through them (or through and over them) to eliminate formation of hazards and damage to private property or public infrastructure.

Regulations

To implement this policy, stormwater management ordinances and design guidelines have been adopted by the City. Requirements vary along each channel reach to recognize the differences related to development conditions, expected increases in flood elevations, and the potential for damages.

b. Recognized Watersheds and Channel Reaches

Watershed Maps

Watersheds are divided into "reaches" to recognize the relationships of geography, land uses, political jurisdiction, and proposed development relative to their effects on existing and ultimate storm flow and flood elevations. Within each watershed, the named regulatory streams are designated as part of the primary system, and individual reaches of each are, in some cases, identified for regulatory purposes.

Watershed Landmarks

A land area is defined as being part of a given watershed if stormwater that falls upon it travels overland by natural or man-made pathways, and enters the main channel of the primary system of that watershed. The primary system and channel reaches are established by physical landmarks such as stream confluences and crossing structures.

Floor Elevations

The elevation of the lowest habitable floor of a structure adjacent to a watercourse of the primary system shall be at least one foot above the base flood elevation associated with the ultimate development condition. However, Table B-2 in Appendix B lists channel reaches where the minimum elevation of the lowest habitable floor of any structure shall be above the base flood elevation by more than one foot. In those cases the minimum floor elevation shall be that shown in Table B-2.

2. Secondary Drainage System

Typical Problems Stormwater problems in the secondary system tend to be localized and scattered throughout the City. Typically they result from inadequate provision for streets, storm sewers, and collection channels. Examples include: excessive ponding in streets at low points, excessive storm flow through principal street intersections, overflow of streets, undersized drainage easements, facilities requiring excessive maintenance & restriction of street uses due to excessive storm flow.

Problem Causes The causes of problems in the secondary drainage system are listed as follows:

- Inadequate capacity for design flows.
- Inadequate allowance for increases in storm flow due to future development.
- No provision for containing and controlling (within designated easements or right of way) the discharge from the 100 year rainfall event under ultimate development conditions.
- Failure to control discharge from new developments that exceeds the capacity of the receiving secondary system, existing or proposed.

Damage or Nuisances The results are creation of nuisance problems and situations where damage to public and private property can occur. Remedial measures may be very difficult to achieve, and may range from expensive public improvement projects to situations where remedies are infeasible from a practical standpoint.

Drainage By Design The policy of the City is to avoid formation of these problems through efforts at the design and development stage. Central to this strategy are the performance standards for drainage design contained in these Guidelines, including the “conveyance pathway” concept for containing the base flood discharge.

Performance Criteria Based on this policy, performance criteria are set for design rainfall events. The emphasis at the performance level is to mitigate the nuisance aspect of storm drainage. An example of a performance standard would be: “design the street and attendant drainage system to carry the discharge from a ten-year rainfall event leaving an area approximately the width of one lane at the center free of any water flow”. These Guidelines contain similar performance standards for various parts of the secondary and primary systems.

Conveyance Pathways The secondary system is to be evaluated and designed for the stormwater conditions that will result for storms up to the magnitude of the 100-year rainfall event based on ultimate development within the applicable basin. From the location where storm flow is first introduced into a public easement or right of way near the upper end of any basin, a “conveyance pathway” shall be identified and provided to a discharge point at a main channel of the primary system. The designated "conveyance pathway" must follow or provide clearly identifiable watercourses. Needs for easements or ROW for conveyance pathways are to be assessed per the provisions of Paragraphs E and F of this Section. The purpose of providing for the 100-year storm level is to prevent the creation of situations hazardous to life, or harmful to public and private property. Accordingly, a major emphasis is on deliberately confining storm flow to designated conveyance pathways.

Watershed Diversion

Generally stormwater emitting from land drained by one named regulatory watercourse of the primary system shall not be diverted to drain into a different named regulatory watercourse of the primary system.

3. Detention / Mitigation

Detention Purposes

Detention is an important mitigation measure. It can be used effectively for either or both of two fundamental purposes. As a tool for watershed management, it can be deployed with other features to minimize potential flooding along major watercourse(s). It can also be used to manage how stormflow is discharged from a property to adjacent properties. Thus, it can be an integral part of stormflow conveyance in route to the primary system or to a tributary thereof. Both are legitimate reasons for using detention facilities and any one detention facility might work toward both purposes, depending on its location in a watershed. The functional purposes for detention are further defined in foregoing Paragraph B3-b of this Section.

a. Detention Requirements

Right Uses

For optimum results detention facilities must be deployed for the right reasons at the right locations. It is the intent of these Guidelines to stipulate the conditions under which detention must be used and why. These Guidelines are not intended to preclude the use of detention at locations where qualified engineers may deem it to be beneficial. Nevertheless, where detention is required by these Guidelines designed facilities must meet the criteria stipulated herein.

Peak Flow Regulated

Where detention facilities are required, peak stormflow rates from a project area resulting from the ten (10), twenty-five (25), and one hundred (100) year storm frequency events shall not be increased at any point of discharge. Regulation of peak flows to allowable levels, as determined by the provisions of these Guidelines, shall be achieved by storage facilities on, or away from, a project area, or by participation in an approved Regional Stormwater Management Program.

b. Detention Facilities May Be Optional

Detention Limited

At the discretion of the City Engineer, land development activity is not subject to the stormwater detention requirements of these Guidelines if one or more of the four conditions listed in Sub-paragraphs 3-b(1) through 3-b(4) before are satisfied, and an engineer registered in the State of Texas submits a signed, sealed, and dated letter addressed to the City Engineer, stating the following without qualification:

> "*I have conducted a topographic review and field investigation of the existing and proposed flow patterns for stormwater runoff from* (name of subdivision or site project*) to the main stem of* (name of creek)*. At build-out conditions allowable by zoning, restrictive covenant, or plat note, the stormwater flows from the subject subdivision or site project will not cause any increase in flooding conditions to the interior of existing building structures, including basement areas, for storms of magnitude up through the 100-year event*":

(1). Adjacent to Primary System

Any development adjacent to the Primary System may demonstrate that detention is not beneficial to the system with an engineering timing analysis. The analysis should include all upstream development broken into basins of size similar to the development being studied and carried downstream until the development represents less than 2% of the total drainage basin.

(2). One Existing Lot
The proposed development project involves one single existing legal lot that is limited to single-family land use by zoning, restrictive covenant, or plat note.

(3). Small Lot
The size of a platted lot is equal to or less than one (1) acre for commercial use, or two (2) acres for detached single family use.

(4). Draining to Designated Streams
At locations included in the drainage watersheds of certain streams stipulated as not requiring detention in Table B-2 in Appendix B, provided Type 2 Detention is not needed for managing property-to-property stormflow.

4. Water Quality

Concurrent Objectives

The intent of these Guidelines is to cause development of stormwater management facilities that effectively collect and convey stormflow without causing water damage impacts on life and property. A concurrent objective is to achieve facilities that minimize any adverse affect(s) on the quality of water conveyed into natural waterways that traverse and/or drain the City.

Water Quality Matters It is important that water quality considerations be integral to all aspects of planning, designing, and constructing any facilities regulated by these Guidelines. When design alternatives are at option, the preferred design will be that offering better water quality characteristics for near-term and long-term conditions, as well as during construction, provided the public safety objectives of these Guidelines are not jeopardized.

Tradeoffs Where tradeoffs are faced between public safety and enhanced water quality in any design, greater favor shall usually be afforded to public safety by the designer. However, consistent with applicable State and Federal regulatory requirements, the City Engineer, or his/her designee, may opt to require greater attention to water quality. All information necessary to such decisions shall be the responsibility of property owners (or applicants) proposing the affective land development project(s).

5. Master Drainage Plans

Plan Consistency All land development projects and site re-development projects subject to the provisions of these guidelines must demonstrate that plans for managing the stormflow expected to emit from the project(s) are consistent with the City's Master Drainage Plan, or with any applicable publicly approved Watershed management master plan.

D. Extent of Design

1. Threshold for Engineered Design

Limited Exemptions For purposes of these Guidelines, some land development projects may be exempted from requirements for drainage plans designed by a licensed engineer and approved by the City. However, in designated FEMA floodplain areas no construction of any kind, including clearing, grubbing or earthwork, may begin without fully approved engineering studies. Likewise, this provision shall not be construed to obviate any requirements of the Texas Professional Engineering Practices Act regarding engineering of facilities to be constructed for public use.

Possible Exemptions Developments of the general nature listed below may be exempted from designs conforming with provisions of these Guidelines after appropriate review and approval by the City Engineer or his/her designee.

- A small lot less than one acre in size that does not receive stormflow from adjacent or nearby land areas.
- A platted lot set aside for construction of one single family residential unit.

- Any platted lot with an area less than one acre for which adequate stormwater management provisions can be administered through building permit requirements.
- Where, in the judgment of the City Engineer, development of a proposed project on a platted lot will have no appreciable down-steam effect**.**

2. Study Limits

Analysis Limits

Engineering for assessment of conditions resulting from a stormwater project shall include the **project area**, **above-project area(s)**, and **pathway area(s)** as necessary, and must extend upstream and/or downstream along designated **conveyance pathways** to a point where the applicant (or his engineer) can demonstrate to the City Engineer's satisfaction that there are no appreciable drainage effects caused by the proposed project. Downstream or upstream of these points the minimum responsibility of the project engineer is to merely document the location of the "conveyance pathway" to limits otherwise specified in these Guidelines.

3. Special / Alternate Designs

a. City Engineer Approval

Equivalent Safe Design

The City Engineer may, upon request, approve an alternate design or construction methodology that differs from the requirements in these Guidelines if the City Engineer determines that:

(1) The alternate design or construction methodology is equivalent or superior to the design that would result from using these Guidelines, and

(2). The alternate design or construction methodology is sufficient to ensure public health and safety.

b. Substantiation of Alternate Designs

Responsibility

It shall be the responsibility of the owner's/developer's (applicant's) engineer to substantiate that any proposed alternate design or construction methodology deviating from these Guidelines meets or exceeds designs or construction methodologies promulgated by these Guidelines.

4. Applicable Ordinance Requirements

Design Reviews

Nothing herein shall be construed to conflict with or supersede design review criteria otherwise established in applicable ordinances of the City of Cibolo.

E. Public Facilities

1. Principles for Public / Private Facilities

Public/Private Mix Stormwater management involves some combination of private and public facilities occurring on (or across) land, and in easements or ROW, in a mix of public and private holding (or ownership). The two-fold intent of these Guidelines is to regulate all such facilities as necessary to achieve specific objectives, while minimizing regulation where it is not fundamental to meeting those objectives.

Rural To Urban Development activities either change the character (or use) of a previously developed site(s), or generally move land from rural to urban conditions. In the later case, storm runoff is necessarily directed into various types of concentrated flow that typically did not previously exist. This can tend to change both how and where flow is delivered to an immediately adjacent property or facility. Because the new facilities are commonly situated in easements or ROW proposed to be conveyed to a public entity, the process may create a measure of public responsibility where none had previously existed.

Discharge Options It is the responsibility of the owner/developer of any development project to properly provide for storm discharge from the project area. Where street or drainage ROW(s) or drainage easement(s) are to be dedicated to the public, and discharge is to drain across neighboring properties before reaching a Named Regulatory Watercourse (or a recognized drainage way serving as a tributary thereof), it shall be the responsibility of the project owner/developer to accomplish one of the two following scenarios, or some combination thereof.

a. First Scenario: Establish Drainage Easement(s)

Receiving Easements Drainage easements must be established across downstream properties as necessary along identified conveyance pathways. Such easements must be aligned and sized to safely accommodate the design discharge(s) from the project area, and must extend to a Named Regulatory Watercourse (or a tributary thereof). The easement(s) may be conveyed to a private party or to a public entity at the discretion of the City Engineer or her/his designee.

b. Second Scenario: Pre-Development Release

Designed Release(s) Drainage facilities must be situated and designed so that discharge(s) are delivered to downstream properties with substantially the same flow characteristics (rate of flow, concentration, velocity, etc.) that existed in pre-development conditions. In addition, discharges are to be released at substantially the same locations that existed in pre-development conditions. Usually, all work necessary to accomplish this must be within the geographic limits of the project area.

2. Maintenance Considerations

A Design Function All stormwater management projects subject to the provisions of these Guidelines that are to be dedicated to the public shall be designed with adequate provisions for maintenance of the designed facilities, regardless of their nature. Maintainability and access are important design objectives. These two factors must be an integral part of the design considerations for all stormwater facilities. The same principles must apply to the easements and/or right of way within which such facilities are to be placed.

Importance Where, in the opinion of the City Engineer, design alternatives meet detention, flood level, and water quality criteria promulgated by these Guidelines and other regulatory requirements in essentially an equal manner, the option(s) offering lesser demand for maintenance work will be preferred. Likewise option(s) offering improved access will be preferred.

Justification Data All information necessary to making such decisions shall be the responsibility of property owners proposing the land development project(s). Changes in proposed designs may be required in order to meet these objectives.

3. Easements and Right of Way

Drainage ROW Where any part of a project area is traversed by a channel or stream, whether man-made or natural, an easement or drainage right of way (ROW) is to be provided for the watercourse. Likewise ROW is to be provided for drainage ways newly formed by runoff concentration within the project area of subdivision projects. In all cases ROW is required unless easements are specifically approved by the City Engineer. ROW will generally be required unless stormflow is conveyed via underground conduit, in which case easements will be considered.

Uses Limited The purpose of easements or right of way (ROW) is to provide the necessary space for stormwater flow and for maintenance of drainage facilities. Any uses of such areas that are inconsistent with these purposes are prohibited. Prohibited uses include, but are not limited to, construction of fences or other obstructions, placement of building structures, or any uses that alter the required shape, configuration, or surface treatment needed for stormwater management functions.

Maintenance Required Day to day maintenance of tall grass and weeds within easements and rights-of-shall be the responsibility of the developer, homeowner association, or other private entity that retains ownership of the property on which a drainage easement or right-of-way is located. The City of Cibolo shall only be responsible for those drainage facilities specifically built for the public and accepted for such function by the City Council.

a. Size Parameters

Approvals Needed Decisions about the necessary alignment and extent of ROW and easements shall be subject to approval by the City Engineer or his/her designee, and shall be based, in part, on drainage information provided by the applicant. Criteria for this determination shall be based on the anticipated amount and spread of stormwater flow, the possibility of increased flow at some time in the future, any concurrent uses to be associated with the designated areas, the space required for the appropriate maintenance equipment and personnel, and the access necessary to conduct maintenance activities.

ROW For Channels Where a land development project is traversed by a constructed swale, a constructed channel, a natural channel, or a stream, drainage ROW conforming substantially to the limits of such watercourse (plus additional width to accommodate flow from a 100-year frequency event) must be provided. Additional width may be required for maintenance purposes.

Conduit Easements Where stormwater is to be conveyed in buried conduits, drainage facilities may be situated in drainage or utility easements provided flow from a 100-year frequency event will be wholly contained within the easement.

b. Minimum Standards

The following minimum standards shall be used in determining the size and placement of drainage easements and ROW.

(1). The minimum width of any drainage easement shall be 15 feet, except where a lessor width is appropriate and is approved by the City Engineer.

(2). For buried conduit storm sewer, the minimum width for any drainage easement (or ROW) that is not congruent with any other pubic ROW or easement shall be 15 feet, and the centerline of the storm sewer shall not be closer than five (5) feet to either side of the easement. In addition, the easement or ROW (inclusive of the conduit capacity) must adequately convey the 100-year storm flow.

(3). For purposes of maintenance access for improved open channels, the minimum ROW width shall be the design top width of the channel plus an additional 20 feet (five feet along one side and 15 feet along the other side). However, where the design top width of the channel exceeds 30 feet, 15 feet of additional ROW shall be provided on both sides of the design channel width. Where special designs approved under the provisions of Section II, Paragraph C3 of these Guidelines will obviate the need for easements of these widths, smaller or narrower easements will be considered by the City. However, in no case shall adequate provisions for maintenance be seriously compromised.

(4). If access to a drainage easement or ROW is not available from public ROW, then an access easement having a width of 15 feet or more shall be provided from a public ROW to the easement or ROW containing drainage facilities.

(5). The width of all easements and ROW shall be sufficient to include areas that will be part of the designated conveyance pathways of the secondary system.

(6). The widths of all ROW for the primary system shall be sufficient to cover the designated floodway for the existing base flood as defined by the latest FEMA regulations.

F. Private Facilities

1. Detention Systems

Guidelines Apply

All stormwater detention facilities required by these Guidelines shall be sized, designed, and constructed in conformance with the criteria stipulated herein and elsewhere in City ordinances or regulations, whether to be retained as private facilities or dedicated to the public within an easement or ROW.

2. Conveyance Systems

Figure II-2

The four conditions described in this sub-paragraph are illustrated in Figure II-2.

a. Discharges Received By Private Land or Facilities

From Private

Stormwater conveyance features that will receive discharge only from private land or facilities at ultimate development conditions may be established as private conveyance systems at the discretion of the City Engineer or her/his designee. Design of such facilities in accordance with provisions of these Guidelines is generally at the discretion of the Registered Professional Engineer in charge of the work.

From Public

Where stormflow is proposed to discharge from existing or proposed public ROW(s) or easement(s) to private land or facilities it is the responsibility of the owner/developer (or applicant) to assure that the project discharge is compatible with the downstream land and conveyance features. This responsibility must be met as outlined in Paragraph E1-a /or Paragraph E1-b of this Section, or via some combination of the two concepts.

b. Discharges Leaving Private Land or Facilities

To Private

In situations where conveyance facilities that are to be permanently in private ownership will discharge to conveyance facilities that are likewise to be permanently held in private ownership, the design is generally at the discretion of the Registered Professional Engineer in charge of the work. At the discretion of the City Engineer or his/her designee, exceptions to this may apply for watershed management purposes.

To Public

Where private lands or facilities will discharge to publicly held lands or facilities, whether in fee simple or in easement(s) or ROW(s), the design, configuration, and construction of the upland facilities shall be in conformance with these Guidelines to the extent required by the City Engineer or her/his designee. Likewise, if private land or facilities are to discharge into floodplain areas or tributaries of a Named Regulatory Watercourse without first traversing public easements or ROW or publicly held land, they are subject to application of these Guidelines at the discretion of the City Engineer or his/her designee.

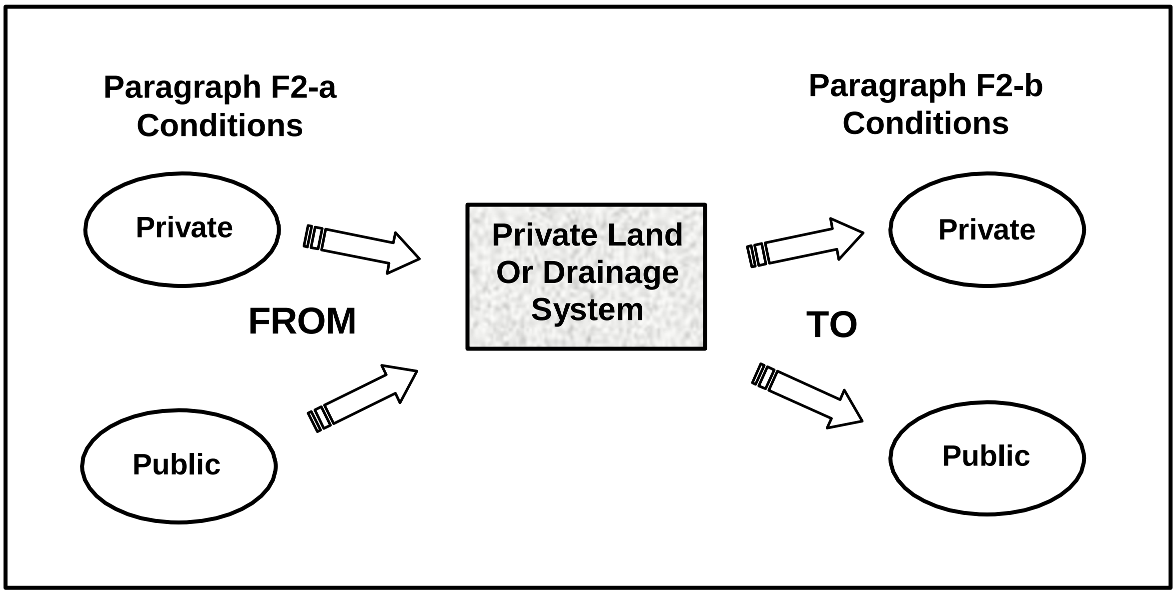


Figure II-2: Public / Private Conveyance Systems Diagram (Paragraph F2)

<u>Section III</u>
Stormwater Administration

SECTION III

ADMINISTRATION

A. Permitting Process

The review process for any drainage plan must be in compliance with requirements of the City of Cibolo as applicable. The following general four-step process is recommended. Depending on the size and hydrologic complexity of the proposed development project, the City may waive one or more steps.

1. Step One

This is a Stormwater Planning Conference with the engineering staff of the City. This may be satisfied in conjunction with a “pre-development conference” or other discussions about any number of other regulatory matters that may affect a particular site or proposed subdivision project.

2. Step Two

A Preliminary Drainage Plan may be required by the City Engineer or her/his designee following the Stormwater Planning Conference. This step has the benefit of formally documenting the questions and decisions reached during the Stormwater Planning Conference. Its review will allow exploration of all drainage issues that may have bearing on a particular project area and will fully identify the drainage study area (those areas requiring some level of identification and/or analysis). This will facilitate expeditious handling of subsequent steps.

3. Step Three

This is submittal of a Drainage Report that fully documents the plan and facilities for managing stormflow of a land development project. At the City’s option this may take the form of an Abbreviated Drainage Plan for smaller projects. In either case this is required for all grading permits, site plans, and subdivision development. The City will provide written notice of review findings pertaining to these reports or plans. This step is completed only when the City has approved the Drainage Report and when engineering plans and specifications for stormwater facilities are “released for construction” by the City.

4. Step Four

The fourth step is filing of a development permit application through which a grading or other construction permit(s) may be issued. The application must be completed by the applicant, and approved by the City, prior to clearing and grading operations on any part of a project area.

B. Stormwater Planning Conference (Step 1)

1. Stormwater Management Concept

Early Discussions

In order to help guide preparation of a plan consistent with City guidelines and minimize work efforts and review time, the design concept for managing storm flow within and from any proposed land development project shall be discussed with the City prior to the development of any specific design, or preparation of construction plans of any kind for drainage facilities. The hydrologic analysis method(s) to be used must be determined and approved as a result of the discussions. The parties representing the proposed development shall obtain all resources, plans, and references necessary to discuss the items outlined in this section. The conference shall address the following information relative to the proposed development.

a. General Location Map

(1). Roadways within and adjacent to the development

(2). Primary and Secondary watercourses and all drainage facilities in the vicinity of a proposed project.

(3). Names, location, and general configuration of surrounding land developments.

b. Project Area Description

(1). Acreage of properties

(2). Location and size of all project phases, if any

(3) Type of land cover (both existing and proposed)

(4). Name of owner and type of development

(5). Current zoning status and proposed change, if any

(6). Any existing natural or man-made topographic features that have the effect of storing or detaining stormwater.

c. Above-Project Areas

(1). Approximate identification of any upland areas that are expected to contribute storm flow to the **project area** (proposed land development project).

(2). Existing and foreseeable future runoff characteristics of all **above-project areas**.

d. Conveyance Pathway Areas

(1) General identification of downstream conveyance pathways for delivery of runoff from the **project area** to a Primary System watercourse

(2) Identification of land areas that generally drain to the **conveyance pathway** downstream of the **project area** and the existing runoff characteristics of those areas**.**

e. Regulatory Watershed Description

(1). Identification of the **Regulatory Watershed(s)** (and Reach thereof) in which the proposed project is located.

(2). General existing land use characteristics of the **Regulatory Watershed**.

(3). References to any available earlier drainage studies that addressed any part or all of the land proposed for development

(4). Applicable Flood Insurance Maps

f. Drainage Basin Description

Thorough Planning

The report should clearly describe the **Basin**(s) of the **Regulatory Watershed** of which the development project is a part. Drainage patterns on both the **project area** and any applicable **above**-**project area(s)** must be clearly identified, along with all anticipated impacts on existing and ultimate development. Likewise, the conveyance pathway(s) must be identified along with **pathway areas** (all areas drained by the conveyance pathway).

(1). General Facility Design

a). The report must identify typical drainage patterns and proposed concepts for managing storm flow generated by the proposed project. This shall include sketch delineation of pathways for conveying stormflow within the **drainage study area** and to the Primary Drainage System.

b). Considerations for handling runoff from **above-project areas**, and to conveyance **pathway areas** must be discussed.

c). The potential need for tables, charts, figures, or drawings to be in the report must be identified.

(2). Specific Details

a). Existing and potential drainage and erosion problems and possible solutions at specific design points must be explored. This is applicable for the entire **drainage study area**, not only the **project area**.

b). The potential need for detention/retention storage must be explored, along with the any proposed outlet design concept.

c). Aspects of the design important to reasonable maintenance access must be identified.

d). Areas to be set aside as drainage easements and/or right of way are to be identified in a general manner.

e). Needs for bridges or culverts for roadway crossing watercourse(s), including any possible need for skewed crossings or watercourse turns at crossings, must be fully identified.

f). All required permits must be identified. This includes those required from the US Army Corps of Engineers, the Federal Emergency Management Agency (FEMA), the Texas Natural Resource Conservation Commission (TNRCC), the Texas Commission on Environmental Quality (TCEQ), TxDOT, or any other State or Federal agency.

e. References

A preliminary list of all criteria, master plans, and technical information applicable to the proposed project must be provided.

2. Preliminary Drainage Plan (Report) (Step 2)

Report is Key

Upon completion of the Stormwater Planning Conference (or the pre-development conference) the City Engineer or her/his designee may require the submission of a Preliminary Drainage Report for the purposes of substantiating any assumptions and/or clearing up any questions identified via the conference. A Preliminary Drainage Report (with Drawings) shall be prepared to generally meet the most salient requirements for the Drainage Report but can be in lesser detail. When a Preliminary Drainage Report is required by the City Engineer (or his/her designee) it shall be submitted and approved prior to substantial preparation of construction plans.

C. Drainage Report Requirements (Step 3)

1. Purpose of Report

Find Needs

The purpose of the Drainage Report is to identify and define conceptual solutions to the problems which may occur as a result of the proposed development, on **project areas, on above-project areas,** and along conveyance areas. The Drainage Report must include drawings as necessary to fully and clearly describe the information required by these Guidelines. All reports shall be printed on 8-1/2" x 11" paper, bound together, and submitted in two hard copies and one electronic copy (pdf format). The report shall include a cover letter presenting the proposed design for review, and shall be prepared by a Registered Professional Engineer licensed in Texas. The report shall contain a sheet authenticating its technical accuracy as follows:

Work Certification

"This report (plan) for the drainage design of (name of development) was prepared by me (or under my supervision) in accordance with provisions of the Cibolo Drainage Design Guidelines

for the owners of the property. All licenses and permits required by any and all state and federal regulatory agencies for the proposed drainage improvements have been issued."

____

Licensed Professional Engineer

State of Texas No.______

(Affix Seal)

2. Abbreviated Drainage Plan

a. Suitability

In certain situations, consistent with the policies and practices of the City, the owner/developer (or applicant) may provide an Abbreviated Drainage Plan in satisfaction of these Guidelines. This is applicable only to small site plan projects on platted lots, not involving the development of stormwater detention facilities, private or public. Although not precluding involvement of an engineer, the scope of such site projects generally does not involve hydrologic or hydraulic engineering analysis or the design of stormwater management facilities. Subdivision land development projects are specifically excluded from this type of submittal. As a function of the size, location, and hydrologic complexity of a project, the City Engineer or his/her designee may require submittal of an engineered drainage report.

b. Submittal Requirements

An Abbreviated Drainage Report is generally a very simple presentation of how stormwater is to be managed on a small project. At a minimum such a plan must include the information listed below. It must be accompanied by a letter of transmittal requesting approval, and all proposed site features must be subject to inspection via building permit processes.

- A site plan drawn to a standard engineering or architectural scale showing vertical dimensional controls and proposed site grading,
- Finish floor elevations of structures and illustration of how stormwater is to be routed around and away from them,
- Illustration of any flumes, walls, berms, and/or landscaping features proposed for the purpose of managing runoff,
- Documentation of how erosion and sedimentation will be prevented as a permanent part of the project,
- Description of how runoff is to be routed away from the property,

- Measures employed to preclude any negative effects on downstream properties, and
- Measures to preclude any negative effects on public or private watercourses to which runoff will be directed.

3. Drainage Report Contents

Report Or Summary

The Drainage Report may be submitted in one of two formats. It may be written in a traditional prose format complete with an executive summary at the beginning, or it may be submitted as a Technical Design Summary. In either format, the report shall be in accordance with the following outline and contain the applicable information stipulated below. The executive summary attendant to a traditional report shall include, at a minimum, the same information as required in Part 1 of a Technical Design Summary, and shall be presented in the same format.

a. General Location and Description of Project Area

(1). Location

a). Streets and roadways within and adjacent to the Project Area (proposed land development project)

b). Named Regulatory Watercourses and facilities

c). Names of existing or approved developments or plats surrounding the proposed Project Area whether adjoining it, or separated from it by a street (or highway) or watercourse.

d). Names and location(s) of master plan(s), preliminary plat(s), and/or site plan(s) for adjoining properties that may be in pending status with the City as of the date of the report, to the extent such information is available from local jurisdictions.

(2) Description of Project Area Property

a). Total acreage of Project Area

b). Acreage of Project Area proposed for near term and any future phased improvements

c). Name of property owner(s) and land developer(s) and applicant (s)

d). Land cover characteristics

e). Primary and secondary system watercourses within or adjacent to the property

f). General description of proposed project

b. Drainage Watershed (s) and Study Area(s)

(1) Regulatory Watershed Description

a). Reference to Named Regulatory Watercourse planning studies such as flood hazard delineation reports and flood insurance rate maps.

b). General existing land use characteristics of the Regulatory Watershed and the applicable Reach(s) thereof.

(2). Drainage Basin(s) (sub-Watershed) Description

a). Identification of drainage flow patterns from above-project areas

b). Impact of proposed development on existing and proposed conveyance pathways to Named Regulatory Watercourse(s)

c). Description of historic drainage patterns in areas proposed for development

d). Description of existing natural or man-made topographic features that have the effect of storing or detaining stormwater within the Project Area.

(3). Drainage Study Area

a). Clear delineation of all of the Project Area (the proposed land development project), all Above-Project Areas contributing, or proposed to contribute, stormflow to the Project Area, and all Conveyance Pathway Areas.

b). <u>Existing</u> drainage conditions and flow patterns for all of the proposed Project Area, and for all Above-Project Areas.

(4). Drainage Plan

a). <u>Proposed</u> drainage conditions and flow patterns for all of the proposed Project Area and for all Above-Project Areas contributing stormflow to the Project Area must be shown.

b). General review of the Conveyance Pathway(s) and identification of any points along it (them) were capacity limitations are known or suspected to exist.

c). General location and size of any proposed detention/retention facilities.

d). Identification of the location and type of all collection and conveyance facilities proposed to serve the Project Area.

c. Drainage Design Criteria

(1). The range of design storm flows anticipated at critical points throughout the proposed drainage system must be shown, in addition to how flow will be accommodated at each point. All assumptions and hydrologic parameters must be shown.

(2). Stormwater Management Criteria Reference(s) and Site Constraints

a). Identification of earlier drainage studies for or including the Project Area or any portion of Above-Project Areas that influence, or are influenced by, the selected drainage design.

b). Demonstration of how conditions in any Above-Project Area(s) will affect drainage design for the Project Area.

c). Explanation of how existing and proposed topographic constraints such as streets, structures, and layout of proposed facilities (including building pads if applicable) will impact plans for managing storm flow.

(3). Hydrological Parameters

a). Documentation for determination of design rainfall

b). Identification of runoff calculation method

c). Identification of detention discharge and storage calculation method, if any

d). Identification of design storm recurrence intervals

(4). Conveyance System Hydraulic Parameters

a). Identification of capacity of various existing and proposed conveyance systems, citing any design or study references used

b). Identification of detention/retention outlet type, if any

c). Identification and explanation of any drainage facility design criteria not presented in these Guidelines.

(5). Any criteria, methods, or design techniques that deviate from these Guidelines must be identified and fully justified.

d. Drainage System Design

(1). General Concept

a). Identification of anticipated and proposed drainage patterns and the proposed stormflow management concept(s).

b). Documentation of compliance with all requirements for managing Above-Project Area runoff in terms of discharge and capacity.

c). Documentation of compliance with requirements for analysis and design of conveyance pathways as determined necessary during the pre-development conference or other meetings with the City Engineer or her/his designee.

d). Explanation of the content of tables, charts, figures, or drawings presented in the report

(2). Specific Details

a). Descriptions of drainage problems and proposed solutions at specific design points

b). Description of detention storage design and outlet design including measures for minimizing erosion at discharge points

c). Identification of access ways for maintenance of all proposed stormflow management features, whether to be privately held or conveyed via platting to the City.

e. Conclusions

(1). Statements of compliance with the Cibolo Drainage Design Guidelines.

(2). Effectiveness of drainage design to control flooding or damage due to design stormflows, including minimization of erosion along conveyance pathways serving the project.

(3). Explanation of the effectiveness of existing and proposed drainage improvements for controlling discharges of the 2-year, 10-year, 25-year, and 100-year storms, assuming ultimate development conditions within the Drainage Study Area of the proposed land development project.

f. References

Reference all criteria, master plans, and technical information applicable to the proposed land development project must be referenced.

g. Appendices (where applicable)

(1). Hydrologic Computations

a). Land use assumptions regarding adjacent properties

b). Minor and major storm runoff at specific design points

c). Runoff computations at specific design points for both existing and ultimate development of all Design Drainage Areas.

d). Hydrographs at critical design points

(2). Hydraulic Computations

a). Culvert capacities

b). Storm sewer capacity

c). Street capacity

d). Storm inlet capacity including inlet control rating at connection to storm sewer

e). Open channel design

f). Detention area/volume capacity and outlet capacity calculations

(3). Municipal Approvals and Permits

This appendix to a drainage report is for the purpose of documenting any approvals or permits issued by the City as applicable. Examples include (but are not limited to) zoning, final or preliminary plats, clearing and grading permits, or building permits. The status of all pending requests is to be documented as well as any issued approvals or permits. Presentation of this information may take the form of a simple list that includes the pertinent identifying data such as case codes, property identification, applicant, and application/action dates. Alternatively, photocopies of application and/or approval documents may be included. Specific requirements for this information should be addressed during the stormwater planning conference.

(4). Non-Municipal Permits

a). Issued or pending permits regarding FEMA-designated Regulatory Watercourses.

b). Issued or pending permits required by the US Corps of Engineers

c). Issued or pending permits regarding water quality or endangered species in stormwater management or land development activities, whether required by units of State or Federal Government.

d). Easements or statements of technical reviews required to satisfy other governmental units.

4. Drainage Report Drawings

a. Sheet # 1 – General Location Map

(1). Depict drainage flows entering and leaving the Project Area

(2). Identify construction along drainage ways, including all areas where natural ground cover is to be removed or significantly disturbed

(3). Illustrate general drainage flow within entire Drainage Study Area

(4). Draw at a scale of between 1' = 500' and 1" = 2000'

b. Sheet #2 – Floodplain Information

(1). Copies of existing 100-year floodplain maps showing the location and approximate boundaries of the land development project.

c. Sheet #3 – Drainage Plan Maps(s) Showing:

(1). Complete Drainage Study Area boundary including: Above-Project Areas and how stormwater flows from them to the Project Area, Conveyance Pathways draining the Project Area, and Pathway Areas.

(2). Entire Project Area, including depiction of areas proposed for near term construction activity, at a standard engineering scale providing complete legibility and on drawings not exceeding 24 inches by 36 inches in size.

(3). Existing and proposed contours at maximum intervals of two feet.

(4). Property lines and easements with purposes noted.

(5). Existing and proposed streets and highways including ROW lines

(6). Existing drainage facilities, roadside ditches, drainage ways, gutter flow directions, and culverts. All pertinent information such as material, size, shape, slope, and location shall also be included.

(7). Boundaries of all Design Drainage Areas.

(8). Proposed type of street flow (roadside ditch and/or gutter flow) and flow directions.

(9). Plan and profile of proposed storm sewers and open drainage ways, including inlets, manholes, culverts, junction structures, and other appurtenances.

(10). Clear indication of changes in pipe size in storm sewer system.

(11). Proposed outfall point(s) for runoff from areas proposed for construction and facilities to convey flows along proposed Conveyance Pathways to outfall points in the system of Named Regulatory Watercourses.

(12). Routing and accumulation of stormflow at various critical points for the minor storm runoff

(13). Path(s) chosen for computation of time-of-concentration.

(14). Location of detention/retention storage facilities and outlet works.

(15). Location and elevations of all documented floodplains affecting the properties proposed for land development.

(16). Location and elevations of all existing and proposed utilities affected by or affecting the drainage design.

(17). Routing of any drainage that must flow through the development project from Above-Project Areas.

(18). Finished floor elevations of existing structures in flood plains adjacent to Primary or Secondary watercourses.

(19). Existing 100-year water surface elevations for each lot or building site in flood plains adjacent to Primary or Secondary watercourses.

(20). Notation about any off-project features influencing the proposed land development.

D. Construction Drawings and Specifications

1. Compliance With Drainage Report

Plans Fulfill Report

Where drainage improvements are to be constructed they must be in accordance with the approved Drainage Report. Construction plans and specifications must demonstrate how and where the stormwater management concepts of the Drainage Report will be implemented. Plans on sheets no larger than 24 inches by 36 inches, together with any specifications not consistent with Cibolo Technical Specifications, shall be submitted for review and approval prior to construction. Plans (plan and profile sheets) and specifications for the drainage improvements will include all of the following information as applicable.

a. Storm Sewer Systems

- Line sizes, alignments, flow line elevations
- Junction boxes, man holes
- Inlets and outlets

b. Culverts

- Size, alignment, flow line elevations
- End treatments
- Inlet and outlet protection

c. Open Watercourses

- Channel alignment, section, and flow line elevations
- Sizes and flow lines of ditches and swales
- Surface treatments

d. Detention Facilities

- Pond size, placement, grading and elevations
- Pond outlets and outfall treatments
- Pilot channel alignment, grade, and section (when used)
- Landscaping

e. Related Structures / Facilities

- Erosion control features
- Provisions for maintenance access
- ROW and/or easements, both public and private as applicable

f. Flood Information

- Finished floor elevations of buildings adjacent to stormwater facilities
- 100-year water surface elevations

g. Approvals

- Engineer's certification
- Action by the City to "release for construction" as evidenced by titles and signatures of required City officials

2. Compliance With Design Guidelines (Step 4)

Thorough Plans

The information presented by the drawings and specifications shall be in accordance with sound engineering principles, the design parameters herein, and requirements for subdivision design stipulated by the City of Cibolo, as applicable. Construction documents shall include geometric, dimensional, structural, foundation, bedding, hydraulic, geotechnical, ecological, landscaping, and other details as needed to construct the storm drainage facilities. The approved Drainage Report shall be included as part of the construction documents for all facilities affected by the drainage plan.

E. Record Drawings

1. Required Plans

Before Acceptance

Plans documenting all constructed public drainage facilities and private detention/retention ponds ("Record Drawings") shall be submitted to the city upon completion of the construction work. These documents (on 24" x 36" and electronic formats) must be received and deemed consistent with all applicable regulations by the City before the improvements will be accepted. The construction drawings are acceptable as Record Drawings provided construction has not significantly deviated from them.

2. Engineering Attestation

Accuracy Of Plans

A registered professional engineer licensed to practice in Texas must attest that the "Record Drawings" provided in satisfaction of the forgoing paragraph are a reasonably accurate representation of the location and characteristics of public storm drainage facilities and all detention facilities (private or public) as actually constructed. The center line alignment within plus or minus six (6) inches, and size of buried conveyance conduit shall be shown. Information about the size, elevation, and conveyance attributes of detention outlet structures and spillways shall be shown. The storage capacity and perimeter elevations of public and private detention ponds shall be shown. Attestation shall be via the following statement affixed with signature and seal to each sheet of the Record Drawings:

"I hereby attest that I am familiar with the approved drainage plan and associated construction drawings and furthermore, attest that the drainage facilities have been constructed within dimensional tolerances prescribed by the Cibolo Unified Stormwater Design Guidelines and in accordance with the approved construction plans or amendments thereto approved by the City of Cibolo.

_______________________________________."
(Cibolo City Engineer)

____
Licensed Professional Engineer
State of Texas No. _____

(Affix Seal)

3. Construction Attestation

Full Construction

Each plan and profile sheet of materials presented as Record Drawings shall bear a certification from the general contractor as follows:

"I certify that the subdivision improvements shown on this sheet were actually built, and that said improvements are substantially as shown hereon. I further certify, to the best of my knowledge, that the materials of construction and sizes of manufactured items, if any, are stated correctly hereon"

____
General Contractor

Section IV
Hydrology

SECTION IV

HYDROLOGY

A. Introduction

Analysis Methods

The two types of hydrologic analyses most often required are the computation of the peak discharge at a specific location and the computation of a hydrograph at a specific location. Two methods are recommended for computation of peak discharges and two methods are recommended for computation of hydrographs. The application of these methods is a function of the purpose of the hydrologic examination and the size of the Design Drainage Areas being examined as outlined in these Guidelines. Other methods of proven use may be submitted to the City for approval. It is highly recommended that approval be obtained before significant hydrologic work is accomplished for a project.

B. Stormwater Runoff Calculation Methods

1. The Rational Formula

a. Variables

The formula shall be expressed as:

$$Q = ciA$$

Where the variables are defined below.

"Q" is the discharge in exact units of acre-inches per hour and accepted to be equivalent to units of cubic feet per second (cfs). This value is taken as the peak or highest discharge expected at a designated design point.

"c" is a coefficient, having no units, that represents the average runoff characteristics of the land cover within the drainage area delineated for a designated design point.

"i" is the rainfall intensity in units of inches per hour (in/hr.).

"A" is the area of land in acres that contributes stormwater runoff that passes through or at a designated design point.

(1). Intensity-Duration-Frequency Relationship

Rainfall intensity (**i**) is defined as the average rate of rainfall in inches per hour. It can be determined for storms of various return frequencies as commonly represented by several intensity-duration-frequency (IDF) curves in graphical form. Duration ranges from ten minutes to 24 hours, and is assumed to be the time of concentration. Rainfall intensities may be determined from (IDF) curves or from the equations presented in Table C-1, Appendix C. These equations approximate the IDF curves within a reasonable margin of error. For the Rational Method, the critical rainfall intensity is that having a duration equal to the time of concentration of the design drainage area. Determination of time of concentration (t_c) is discussed in Paragraph B1-a(3) below.

2). Runoff Coefficients

a). Tables C-2 and C-3 in Appendix C shall be used to select the runoff coefficient "**c**" for the appropriate land cover and land use. Linear interpolation shall be used to choose specific values within the ranges given.

b). For areas that consist of different types of land cover or land use, a weighted average runoff coefficient shall be computed using the following equation.

$$c = \frac{c_1 A_1 + c_2 A_2 + \ldots c_x A_x}{A}$$

Where:

$A = A_1 + A_2 + \ldots = A_x$ the total drainage area,

$c_1, c_2, \ldots c_x$ are the runoff coefficients for sub-areas,

$A_1, A_2, \ldots A_x$ are the areas of land cover or land use that correspond to the runoff coefficient **c_1, c_2 … and c_x** respectively, and

c is the runoff coefficient for use in the formula for the Rational Method.

c). The runoff coefficient "**c**" shall be determined using the "land use" when using the rational formula to compute the peak discharges within or from specific sites and developments.

d). Referring to Tables C-2 and C-3 in Appendix C, the runoff coefficient "**c**" may be determined from the "land use" when using the rational formula to compute the peak discharge from more than one site or development.

(3). Time of Concentration

a) Principles --Time of Concentration (t_c) is the theoretical time required for a drop of rain to travel from the most hydraulically remote point in a Design Drainage Area to a point where storm flow is to be determined (the point of calculation). Assuming rainfall is uniform over time and uniform on the watershed, the time of concentration is the first moment when the entire Design Drainage Area is contributing to the runoff at the point of calculation, because during that time all other parts of the Design Drainage Area will also be contributing flow to that point. This is fundamental to estimating total flow at the point of calculation based on the assumption of uniform rainfall over time, as accomplished using the Rational Method.

Hydrograph Peak

When used within computations using shaped unit hydrographs, the time of concentration is used (usually indirectly) to determine the timing of the peak of the hydrograph in relation to the beginning of the storm event.

Watershed Factors

The length of time will depend on several characteristics of the Design Drainage Area. Slope, ground cover, degree of concentration, and the antecedent moisture content of the soil are principle among these. When such characteristics are not entirely uniform it is necessary to assess the composite effects of differing characteristics found in parts of the Design Drainage Area. Because hydraulic equations are rarely linear in nature, the averaging of characteristics, such as slope, can readily create inaccuracies. Likewise, multiple variations in characteristics of the Design Drainage Area can cause compounding of inaccuracies, thus generating unreliable results.

Segment Analyses

In order to ensure accurate results, each segment having different characteristics must be calculated independently, and the resulting times then added to obtain the overall time of concentration ($\mathbf{T_c}$). The time of concentration should be determined for each segment of significantly differing slope, surface roughness, and/or cross sectional area. Values of velocity (**v**) for determining (t_c) for each segment are given in Table C-4 in Appendix C. The time needed for runoff to flow through each of these segments is known as Travel Time ($\mathbf{T_t}$).

Flow Characteristics

To expedite these calculations, formulas have been developed to estimate travel time by factoring out certain variables from the basic hydraulic equations. Some are assumed to be effective for the initial sheet flow state where the runoff is spread very thinly over a relatively wide area. Some equations are applied to a condition known as 'shallow concentrated flow' in which the runoff is not in a uniform sheet, but is concentrated in an irregular manner not allowing determination of flow cross sections. Where flow is channelized in a reasonably uniform manner allowing use of cross section information, Manning's Equation is normally used to determine velocity, and thus time of travel.

b). Analysis Criteria -- For purposes of consistency, these Guidelines provide a single set of equations for the estimation of Time of Concentration. These equations and related criteria are adapted directly from the TR-55 manual published in 1986 by the Soils Conservation Service (now the Natural Resources Conservation Service). Other accepted methods may be submitted and considered as special designs.

Initial Sheet Flow: For initial flow areas, which are both uniform and planar, Manning's Kinematic equation (shown below as published by Overton and Meadows, 1976) should be used. Its use is based on the four assumptions listed below. In no case should a length exceeding 300 feet be considered.

- Shallow uniform steady flow
- Constant rainfall intensity
- Rain duration of 24 hours
- Infiltration does not impact travel time

$$T_t = \frac{0.007(nL)^{0.8}}{(P_i)^{0.5} s^{0.4}}$$

Where:

T_t = Travel time (hours)

n = Mannings' roughness coefficient for sheet flow (Table C-5, Appendix C).

L = Overland flow distance (feet)

P_i = Recurrence interval for the 24-hour rainfall depth (inches) in the **i^{th}** year (Table C-6, Appendix C)

S = Slope of land (feet per foot)

Shallow Concentrated Flow: For reaches where the flow is no longer uniform and planar, and a flow cross section cannot be determined, the equation for shallow concentrated flow should be used. This equation estimates flow velocity, which can be translated into travel time.

$$T = \frac{D}{60V}$$

Where:

T = Travel time (minutes)

D = Flow distance (feet), and

V = Average velocity of runoff (feet per second)

Channel Flow: Where a flow cross section can be determined, Manning's Equation should be used with appropriate factors for the segment being analyzed.

In any case the time of concentration need not be taken as being less than 10 minutes.

b. Assumptions and Limitations

(1). The Rational Formula shall only be used to estimate peak discharges at specific designated design points.

(2). The contributing area **"A"** of runoff shall not exceed 50 acres.

2. Natural Resource Conservation Service (formerly SCS) Methods

a. Hydrology Principles

"SCS" No. 55

Technical Release No. 55 – Urban Hydrology For Small Watersheds forms the basis for examination of watersheds considered large as regulated by these Guidelines. These "SCS" methods are empirically derived relationships that use precipitation, land cover, and physical characteristics of Design Drainage Areas to calculate runoff amounts, peak discharges, and hydrographs. Of the various methods available, the following two are adopted for use:

(1). Chart Method – used to determine the peak stormwater discharges and the effect of development on those peak discharges at a designated design location.

(2). Tabular Method – used to determine a hydrograph of stormwater discharges at a designated design location.

b. Variables

(1). 24 Hour rainfall depths for the Cibolo area (Table C-6 in Appendix C) shall be used to select the rainfall depth for selected storm return periods. This value shall be used for the variable "P" as input to all equations, graphs, and tables as applicable. A Type III rainfall distribution developed in 1990 shall be used to determine incremental totals.

(2). Hydrologic Land Cover Parameters (SCS Curve Numbers)

a) The engineer shall determine the land cover parameters based on soil type from the county soils maps and natural vegetation only. All development shall be input as impervious percentage per Table C-7.

(3). Determination of Peak Discharges – The TR-55 Chart Method

a). Calculations must include the appropriate factors and modifications for the shape and slope of the Design Drainage Area, and urbanization (percent of impervious area and percent of hydraulic length modified).

b). Where a Design Drainage Area consists of several types of land cover and/or land use, a composite percent of impervious area shall be determined using the same methodology outlined in Paragraph B1-a-(2)-b) of this Section.

(4). Determination of Time of Concentration

One of two methods shall be used, the "Lag Method" or the "Upland Method". Details on the use of both are available in "TR-55".

c. Assumptions and Limitations

(1). The accepted methods from Technical Release No. 55 are for use in determining stormwater discharges and hydrographs in the Secondary Drainage System only.

(2). Application of these methods shall be in strict conformance with the instructions and recommendations given in Technical Release No.55 and the latest updates and revisions issued by the Texas Natural Resource Commission (formerly the SCS), except as superseded or altered by the requirements of this section.

(3). The Design Drainage Area for application of these methods shall not exceed 2000 acres.

3. Hydrograph Methodology

a. Methods

Hydrographs

Two methods of determining a hydrograph are accepted for use. These are the Tabular Method of NRCS (formerly SCS) Technical Release No. 55, and the NRCS (formerly SCS) Dimensionless Unit Hydrograph method. The principal aspects of each are outlined below.

(1). Tabular Method of NRCS (SCS) Technical Release No. 55 --The hydrograph is computed by an empirical method that relates drainage area, land use, and time of concentration.

(2). NRCS (SCS) Dimensionless Unit Hydrograph – The hydrograph is computed using basin area, land cover, lag, and precipitation as modifiers to a dimensionless unit hydrograph.

(3). Combining Hydrographs – In larger Design Drainage Areas covering large Basins or entire Watersheds it may be necessary to combine hydrographs in order to accurately depict the runoff with one hydrograph where two or more sub-areas intersect and combine flows. If this occurs, the drainage report shall explain the location of these intersections, and provide the necessary input files in conjunction with the report.

b. Assumptions and Limitations

(1). Tabular Method of NRCS (SCS) Technical Release No.55

a). This method shall be applied according to the instructions and limitations outlined in NRCS (SCS) Technical Release No. 55, and revisions issued by the Natural Resource Conservation Service.

b). This method shall only apply to analysis of the Secondary Drainage System.

(2). NRCS (SCS) Dimensionless Unit Hydrograph Method

a). This method is used in the hydrologic analysis for the adopted Flood Studies of the City.

b). The method shall be used to compute hydrographs at locations in the primary system where the adopted Flood Study does not determine a hydrograph.

c). The method shall be applied using the Generalized Computer Program, HEC-HMS, Flood Hydrograph Package developed by the Hydrologic Engineering Center of the U.S. Army Corps of Engineers. At the discretion of the City Engineer the HEC-1 Program may be used.

d). Data from the adopted Flood Study shall be used with only the modifications necessary to account for the desired location of the hydrograph. This will typically involve deletion of data for areas outside of (or downstream of) the study location, and modification of the most downstream drainage area and/or routing reach.

c. Computer Analysis and Simulation

(1). A comprehensive hydrologic model of several of the Primary Systems has been adopted by the City. Most of the models are applied using Generalized Computer Program, HEC-1, Flood Hydrograph Package of the U.S. Army Corps of Engineers.

(2). The model uses the following methods available in HEC-HMS:

a). Precipitation is computed using the 24 hour rainfall depths (see Table C-6 in Appendix C) distributed according to the Natural Resource Conservation Service Type III Distribution.

b). Basin Hydrographs are computed using the NRSC (SCS) Dimensionless Unit Hydrograph Method.

c). Routing of hydrographs is computed by Normal Depth Storage and Outflow (“Channel Routing”).

(3). Amendment of the adopted FEMA flood study will be processed by the City as conditions in the drainage basins change based on revised flood study data submitted to the City for review.

Range of Analyses

The model consists of analyses of the 10-year, 25-year, & 100-year storms for two Design Drainage Area conditions: ”Existing” and “Ultimate”. The “Existing” condition analysis reflects the land uses and channel conditions in the Design Drainage Area as they exist at the time of analysis. The “Ultimate” condition analysis reflects the fully developed conditions defined by the adopted Comprehensive Land Use Plan guiding development within the City, coupled with the existing channel and floodplain conditions at the time of the study. No allowance is to be made for proposed channelization in determining the “Ultimate” condition flood discharges or elevations.

C. Applications

1. The Rational Method

Limited Use

The Rational Formula shall be limited to use in determining the peak discharge from small areas of overland or sheet flow, and concentrated flows in street gutters, storm sewer, and man-made channels. It shall not be used for determining peak discharge from any Design Drainage Area exceeding 50 acres in size nor for determining or estimating storage or discharge requirements for design of detention facilities. Likewise it shall not be used to estimate stormwater discharges of the primary system. Its use is strictly limited to small Design Drainage Areas discharging to the secondary drainage system.

2. Natural Resource Conservation Service (NRCS) Methods

Primary Use

Methods promulgated by the NRCS (formerly the Soil Conservation Service – SCS) have a variety of applications. Those detailed in Technical Release No. 55 are for use in determining stormwater discharges and hydrographs in the Secondary Drainage System only and for Design Drainage Areas not exceeding 2000 acres. For purposes of these Guidelines these methods are applicable to Design Drainage Areas of 50 to 2000 acres. In the event a Design Drainage Area exceeding 2000 acres is to be analyzed, the methodology must receive specific approval of the City engineer.

3. Dimensionless Unit Hydrograph Method

This method must be used where analysis and design of the primary drainage system is involved.

4. Detention Facilities

Storm flow hydrographs for use in designing detention facilities shall be determined using one of the methodologies defined in Paragraph B3 of this Section. The applications and limitations therein stated shall apply.

<u>Section V</u>

Hydraulic Design

SECTION V

HYDRAULIC DESIGN

A. Street Drainage

1. Design Principles

Street Purposes

The primary purpose of streets is transportation: to offer effective mobility for all users, and to ensure that each land parcel has reasonable access. Stormwater collection and conveyance is an important, but secondary purpose. Consequently, designs for handling storm flow should minimize interference with transportation uses. In general, the more important the street (in terms of functional classification) the more important it is that stormwater design not interfere with transportation uses. Conversely, moderate interference with transportation uses is more acceptable on lower class streets.

Flow Parameters

The design flow of water in streets shall be related to the extent and frequency of interference with traffic as related to street functional class and the chance of flood damage to surrounding properties. Interference with traffic is regulated by design limits of the spread of water into traffic lanes. Flooding of surrounding properties is regulated by limiting the depth of flow at the curb and by containment of the 100-year design storm flow within the street right of way.

2. Performance Standards and Limitations

a. Velocity of Flow

(1). The maximum velocity of street flow shall not exceed 10 feet/second. At "T" street intersections flow velocity must be checked on the stem of the "T" to ensure that flow will not traverse the crown and opposing curb of the crossing street and enter onto private property.

(2). A minimum velocity shall be maintained to ensure cleansing flushes at low flows by keeping the minimum gutter slope to six tenths of one percent (0.006 ft/ft).

b. Allowable Depth of Flow

Top of Curb

The depth of flow shall be limited to the top of curb for a design storm having a return period of ten years.

Within ROW

Design flows for storms with an average return period up to and including 100 years shall be confined within the limits of the street right-of-way until discharged into a drainage easement or drainage ROW that is part of the designated Conveyance Pathway system, or directly into a main channel of the primary drainage system. The capacity of the storm drain system shall be increased beyond other design criteria in these Guidelines as necessary to ensure this objective. Design computations shall demonstrate satisfaction of this criterion.

c. Grades and Cross-slopes

Street grades and cross-slopes shall be consistent with all City of Cibolo UDC and Design and Construction Manual specifications.

d. Allowable Water Spread

(1). Local Streets – The design storm flow in local streets shall be limited to the top of crown or the top of curb, whichever is less. Stormwater shall be removed from the streets by inlets or openings into adjacent drainage systems. These shall be placed at low points and as frequently as necessary to avoid exceeding water spread and depth criteria. The design storm shall have a return period of ten years.

(2). Collector Streets – Design storm flow in collector streets shall be limited so that one 12-foot wide area (one traffic lane width) at the center of the street will remain clear of water. Stormwater shall be removed from the street by inlets or openings into adjacent drainage systems. These shall be placed at low points and as frequently as necessary to avoid exceeding water spread and depth criteria. The design storm shall have a return period of ten years.

(3). Arterial and Parkway Streets – Design storm flow in arterial and parkway streets (any street having a raised median regardless of classification) shall be limited so that one (1) twelve-foot traffic lane each direction at the center of the street (or one on each side of a raised median) will remain clear of water. Stormwater shall be removed from the street by inlets or openings into adjacent drainage systems. These shall be placed at low points and as frequently as necessary to avoid exceeding water spread and depth criteria. The design storm shall have a return period of twenty-five years.

(4). Intersections – Inlet placement and storm sewer size shall ensure that design storm flows are intercepted ("dried up") along street legs entering the intersection in advance of the curb returns connecting the streets based on the criteria provided below. In no case shall inlets be placed in the curved portion of curbs connecting intersecting streets. Where storm flow is allowed to pass through an intersection, valley gutter design must provide for smooth, uninterrupted traffic flow as stipulated by City of Cibolo Design & Construction Manual specifications.

Intersection Pair	Intercept	Valley Gutter Criteria
Arterial – Arterial	All legs	No valley gutters
Arterial – Collector	All legs	No valley gutters
Arterial – Local	All legs	No valley gutters
Collector – Collector	All legs	No valley gutters
Collector – Local	Local legs	Valley gutters can parallel Collector
Local – Local	Two legs preferred	Valley gutters acceptable

(5). Mid block Cross Drainage – Where storm drainage is collected on one side of a street and must be conveyed to the other side, it shall be accomplished via underground conduit unless the roadway is functionally classified as a local street. Where storm flow is to cross such a local street the preferred conveyance is via underground conduit, however, at the discretion of the City Engineer, very low design flow may be conveyed in a valley gutter that satisfies all Cibolo Design and Construction Manual specifications.

3. Design Procedure

a. Straight Crowns

Flows in streets which have a straight crown will be calculated using the following equation for triangular channels:

$$Q = 0.56 \frac{z}{n} S^{0.5} Y^{2.67}$$

where,

Q = gutter discharge (cubic feet per second)

z = reciprocal of the crown slope (ft/ft)

S = street or gutter slope (ft/ft)

n = Manning’s roughness coefficient

Y = depth of flow (ft)

When flows over concrete or asphalt pavement are being calculated, the valve of “**n**” shall be taken as 0.018.

b. Parabolic Crowns

Flows in streets which have a parabolic crown become complicated and difficult to precisely solve for each design case. Design equations must be used to determine gutter flow when street design is to include parabolic crown sections. If parabolic crowns are planned, the concept is to be discussed during a Pre-application Conference with the City Engineer or her/his designee.

B. Storm Drain Inlets

1. Principles

The purpose of a storm drain inlet is to intercept street or surface runoff and direct it into another component of the drainage system, usually an underground conduit. Inlets are typically of the curb opening type for streets and grate type for area drainage. Curb inlets occur at low points or on grade, and can have a throat that is either depressed or flush with the gutter invert grade. Grate inlets can only occur in low points and may or may not be depressed.

2. Street Inlet Criteria

Recessed Inlets

Inlets along arterial or major collector streets shall be recessed (horizontally displaced) away from the line of the curb so that any depression at the mouth of the inlet occurs wholly within the limits of the gutter, with no irregularity of elevation extending into the travel lane. A diagram of a recessed inlet is illustrated in Figure C-1, Appendix C.

Inlets on minor collector streets shall be recessed away from the line of the curb when a depression of four (4) inches or greater is used at the mouth of the inlet.

Optional Design

Inlets along streets classified as “local” may or may not be recessed.

Inlet Length

Curb opening inlets shall have a minimum length of five (5) feet, and construction details shall conform to the City of Cibolo Design and Construction Manual specifications.

3. Types of Inlets

Standard Inlets

Standard inlets are classified into two groups: inlets in sumps (Type A) and inlets on grade (Type B). These are further subdivided as follows:

<u>Inlets in Sumps</u>

- Curb openings (with or without gutter depression) Type A-1
- Grate inlet; Type A -2

<u>Inlets on Grade</u>

- Curb openings with gutter depression Type B-1
- Curb openings without gutter depression Type B-2

Combination Inlets

A combination inlet is a side-by-side placement of a standard curb inlet and a grate inlet. The upstream inlet may be a standard curb inlet or simply part of an inlet. The benefit is that the curb opening tends to intercept debris that might otherwise clog the grate inlet. Such arrangements typically offer very little additional capacity over standard depressed inlets. In order to determine the capacity of a combination inlet on grade, it is recommended that the capacity of each (standard and grate) be calculated and the greater capacity be assumed for the pair for design purposes.

4. Inlet Location

Limit Conflicts

Inlet locations shall conform to the requirements of paragraph A of this section of these Guidelines, and shall be located as feasible to limit conflicts (caused by the inlet itself or associated stormwater) with vehicle, bicycle, or pedestrian traffic.

Limit Cross-Flow Inlets shall be located along streets to prevent concentrated stormwater flow from crossing traffic lanes, except as outlined in paragraph A of this section. Typical locations for these conditions are at transitions to super elevated sections, at the ends of long traffic islands, or at the ends of medians in super elevated sections.

Meet Standards Specific configuration and exact location of inlets shall be consistent with requirements of the B-CS Technical Specifications but shall not be in conflict with provisions of Paragraph A2-d of this Section.

5. Inlet Sizing

a. Inlets in Sumps

Minimize Ponding These inlets are placed at low points to relieve ponding of surface water. For purposes of design, inlets having a gutter depression greater than five (5) inches on streets with less than a one percent (1%) grade may be considered as inlets in sumps.

Maximum Depth Under no circumstances shall inlets at low points in streets allow water to pond to a depth exceeding 24 inches above the gutter flow line for up to 100-year frequency design storms based on project buildout and ultimate development conditions. Where computations show that this would be exceeded, provision must be made for an overflow outlet designed to handle the excess flows. This can take the form of a flume draining the street or a swale in an adjacent drainage easement, provided neither present an obstruction to non-motorized travel. Alternatively, the inlet system and receiving facilities shall be oversized as necessary.

(1). Curb Openings Inlets (Type A-1) that are not submerged are considered to function as a rectangular weir with a discharge coefficient of **3.0**. The capacity of a curb opening inlet is found by the following equation:

$$Q = 3.0Ly^{1.5}$$

where:

Q = capacity in cubic feet per second (cfs)

L = length of the opening which water enters into the inlet

y = total depth of water or head on the inlet

Clogging Factor Because of the tendency for curb opening inlets in sumps to collect debris, their calculated capacity shall be reduced by ten percent (10%) to compensate for potential clogging.

(2). Grate Inlets (Type A-2) are considered to function as an orifice with a discharge coefficient of 0.60. The capacity of a grate inlet is based on the following equation:

$$Q = 4.82 A_g y^{0.5}$$

where:

Q = capacity in cubic feet per second

A_g = clear opening area in square feet

y = total depth of water or head on the inlet in feet.

Clogging Factor

Because of the tendency for grate inlets to collect debris, their calculated capacity shall be reduced by twenty-five percent (25%) to compensate for potential clogging, except where used as a controlling device in a detention facility.

b. Inlets on Grade

(1). Curb Inlets (without gutter depression) Type B-1

The capacity of such inlets is based on the weir equation, reduced to account for street grade and cross-flow effects. The head, "**y**", shall be taken as the depth of flow at the upstream end of the opening determined via criteria stipulated in Paragraphs A2 and A3 of this Section. Equation 1 in Table C-8 (Appendix C) shall be used to determine the capacities of these inlets on grade, with the value for "**a**" set equal to zero.

(2). Curb Opening Inlets (with gutter depression) Type B-2

The same guidelines and criteria apply as for those inlets without a gutter depression, except the value "**a**" shall be taken as the gutter depression. The gutter depression is defined as the difference in elevation from the normal gutter grade line to the pavement grade at the throat or entry of the inlet (see Figure C-2 in Appendix C).

(3). The equations in Table C-8 in Appendix C are to be used to determine the necessary size of curb inlets on grade. The applicable determinates and variables are defined in the table and the purpose of each equation is described.

C. Storm Drainage Systems

1. Principles

Conduit System

Storm Drain systems are conduits for the collection and conveyance of surface water to desired points of discharge. Design is accomplished by application of the Manning equation either directly, or through charts and nomographs derived from the equation. The following general conditions apply to the design.

Accept Design Flow The system must be designed to accommodate all intercepted flow for the design storm at each inlet and opening that allows stormwater into the system. Preferably the system shall operate "flowing full" and within the theoretical limits of open channel flow for the required design flows.

Future Runoff Design and construction shall take into account any stormflow from future subdivision areas contributing to the system. No existing system shall have flows added (or directed to it) that will exceed its theoretical design capacity.

100-Year Runoff The system shall be evaluated with associated drainage systems for the flow conditions that will result from a 100-year frequency rainfall event under ultimate development conditions over the Design Drainage Area. Design shall be revised as required to prevent formation of any conditions that could be considered hazardous to life, property, or public infrastructure, or that could create conditions inconsistent with the requirements of other sections of these Guidelines.

2. Initial Design Considerations

a. Minimum and Maximum Velocities

Minimum velocities are necessary to prevent excessive deposits of sediment that could lead to clogging. The minimum design velocity for conduits flowing full shall be 2.5 feet per second.

Maximum velocities are necessary to prevent excessive erosion of the inverts. The maximum design velocity for conduits flowing full shall be 15 feet per second.

b. Roughness Coefficients, "n"

Selection of a roughness coefficient should reflect the average condition present during the life of the conduit. Factors to consider are erosion of the interior surface, displacement of joints, and introduction of foreign material and deposits. The following values shall be used for the materials listed:

Reinforced Concrete: 0.013

Ductile Iron or steel (Smooth): 0.010

Corrugated Metal: 0.024

Smooth lined High Density Poly-Ethylene (HDPE): 0.012

Non-lined High Density Poly-Ethylene (HDPE): 0.020

c. Location of Manholes and Junction Boxes

(1). Junction boxes shall be provided at all changes in conduit size and grade, and where changes in alignment are made at pipe joints

Manhole access shall be provided as part of the design of all junction boxes unless otherwise approved by the City Engineer.

(2). Manholes shall be provided at intervals not to exceed 300 feet for conduits 54 inches in diameter or smaller. For conduits exceeding 54 inches in diameter, the interval between openings shall not exceed 500 feet.

d. Minimum and Maximum Grades

(1). The minimum grade for conduits shall be that necessary to produce the minimum acceptable velocity per Paragraph C2-a.

(2). In order to prevent formation of a hydraulic jump conditions at the terminus of a conduit, the maximum grade along the outfall shall be less than the calculated grade that would result in supercritical flow, except where approved energy dissipation measures are used.

e. Minimum Pipe Diameter

18-Inch Usual

In most instances conduit that will become an integral part of the public storm sewer system shall have a diameter of 18 inches or greater. For design purposes, conduits having a diameter of 24 inches or less shall be assumed to have a twenty-five percent (25%) reduction of cross-sectional area to compensate for potential partial blockage.

Limited 12-Inch

At the discretion of the City Engineer, short laterals connecting inlets to a main line, and the last run of conduit at the uppermost end of a main line, may be twelve (12) inches in diameter. In no case shall a run of twelve-inch conduit serve more than one inlet or exceed a length of 30 feet.

f. Other Considerations

Designs shall attempt to increase the velocity in the direction.

Pipe sizes shall increase in the downstream direction, regardless of additional capacity developed by increased grade, and pipe soffit (inside top) elevations shall be aligned.

An elevation drop is to be provided at all inlets, manholes, and junction boxes equal to the change in pipe diameter or a minimum of one tenth of a foot.

Pipe shall be placed on design friction slopes as much as practical.

3. Hydraulic Design Requirements

a. Flow Assumptions and Manning's Equations

Design shall be by application of the Continuity equation and Manning's Equation as follows:

$$Q = AV$$

$$Q = \frac{1.49}{n} AR^{0.67} S_f^{\ 0.5}$$

where :

Q = flow in cubic feet per second

A = cross sectional area in square feet

V = velocity of flow in feet/sec

n = roughness coefficient of conduit

R = hydraulic radius = **A/WP** in feet.

WP = wetted perimeter in feet
S_f = friction slope of conduit in feet/foot

Capacity of a given size conduit is based on an assumption that it is "flowing full". Thus, **R** is equivalent to the cross sectional area divided by the inner circumference, while a value for **n** and **S_f** must be chosen.

b. Head Losses and Friction Losses

Head losses computed at junctions, inlets, and manholes shall be determined using the following equation:

$$h_j = k_j\left(\frac{V_2^2 - V_1^2}{2g}\right)$$

Where:

h_j = head loss in feet at structures

V_1 = velocity at upstream entrance of structure (feet per second)

V_2 = velocity at downstream exit of structure (feet per second)

k_j = structure coefficient of loss (Table C-9, Appendix C)

g = 32.2 feet per second per second

Head losses due to friction for open channel flow conditions are found by the following equation:

$$h_f = S_f L$$

where:

h_f = head loss due to friction in feet

S_f = friction slope (normally equal to the slope of the conduit, S_o), in feet per foot

L = length of conduit in feet

c. Computation of Hydraulic Grade Line

All designs shall verify the elevation of the hydraulic grade line by calculation along the length of the system for two conditions. For the 10 year design storm the theoretical hydraulic grade line shall be verified as being at least one half foot (0.5 feet) below the inlet opening elevation, the gutter elevation, or the ground surface whichever is lowest. The hydraulic grade line shall also be calculated for the 100-year frequency storm assuming ultimate development conditions in the Design Drainage Area, and must be kept within the limits specified in all other sections of these Guidelines.

d. Allowance for Surcharging

Design of the system and evaluation of hydraulic grade lines shall take into account the tail water elevation at the outlet or final discharge point. Discharge at free outfalls shall assume a starting water surface elevation at the soffit of the conduit. For outlets that might operate in a submerged or partially submerged condition, the starting water surface elevation shall be taken as the water surface elevation of the receiving facility at that location or the conduit soffit, whichever is highest.

4. Use of WINSTORM Program

Use of the WinStorm computer program is acceptable for calculating the capacity of inlets and storm drain systems. The program is available at no cost through TxDOT's web site. If WinStorm is used as a design aid for a project, the complete report the program can generate shall be submitted as part of the drainage report. In addition, both an analysis layout and an electronic medium of the analysis shall be provided.

D. Open Channels

1. Principles

Analysis of open channels is necessary to determine the depth and velocity of a given flow for an established cross-section. Typical uses are to determine the tail water and/or the back water condition(s) at a culvert structure, flood elevation for selected discharge of natural streams and watercourses, and discharge capacities for existing or proposed designed channels.

Design Objectives

Design of open channels involves the selection of a cross-section, surface treatment, and alignment to accommodate some series of design discharges. A successful channel design can take one of two basic forms. It can replicate the features and characteristics commonly found in natural streams, or it can provide the characteristics of traditional constructed channels. In either case the design objective is to provide stable structural components that will not develop excessive sediment deposits or erosive cuts, that will maintain a stable cross-section, that will minimize the need for maintenance, and that will not be damaged by entry of uncontrolled surface flows.

Natural Designs

Leaving streams in their natural state offers numerous advantages, so this practice is preferred. Designs that replicate the characteristics of natural streams are encouraged, provided they meet the objectives of the provisions in these Guidelines. Such a design approach may be required at the discretion of the City Engineer. Where plant growth and hydro-environments can be created or maintained to accomplish stabilized channels they are encouraged. Such designs must ensure that long term maintenance costs are not likely to be greater than would be expected from the use of traditional channel lining treatments.

2. Determination of Water Surface Profiles

a. Methods of Analysis

(1). Manning's Equation

The equation is expressed as follows:

$$Q = \frac{1.49}{n} A R^{0.67} S^{0.5}$$

where:

Q = the discharge in cubic feet per second

n = Manning's Roughness Coefficient

A = cross-sectional area representing the depth of flow in feet

R = hydraulic radius = **A/WP** in feet.

WP = wetted perimeter of channel section for area "**A**" in feet

S = slope of channel bed in feet/foot

The equation is applied to a single cross-section and assumes a uniform channel cross-section and slope as well as steady, uniform flow in the channel. Consequently, its use shall be limited to designed channels and suitable natural channels in the secondary drainage system.

(2). Standard – Step Procedure

This procedure shall be used in analyzing natural or man-made channels of the primary drainage system. It may also be applied to open channels in the secondary drainage system.

Bernoulli's Equation

The procedure involves application of Bernoulli's Equation to a series of stream cross-sections using the continuity equation, the velocity head, and Manning's Equation as inputs. A detailed description is beyond the scope of these Guidelines.

HEC-RAS Software

The method shall be applied using the HEC-RAS software endorsed by the Hydraulic Engineering Center of the U.S. Army Corps of Engineers, or other computer analysis programs employing the same methodology. The application shall be according to the recommendations contained in the user's manual for the program.

b. Primary Design Parameters

(1). Channel Section

Cross-section(s) should be representative of the channel reach being studied.

(2). Manning's Roughness Coefficients ("**n**" values)

Section of values for "n" shall fall within the range of values and descriptions given in Table C-10 in Appendix C.

(3). Channel Slope

The slope of the channel shall be taken as the average slope along the reach being studied.

c. Determination of Flow Character

In order to prevent formation of areas of supercritical flow and hydraulic jumps except where planned, flow must be kept within the limits of sub-critical flow. To do this, design flow depth must be greater than critical depth. For non-rectangular channels, the critical depth can be found through application of trial depths and the following relationship:

$$\frac{Q^2}{g} = \frac{A_c^3}{T_c}$$

where:

Q = discharge in cubic feet per second

g = 32.2 feet per second per second

$\mathbf{A_c}$ = cross-sectional area of flow at critical depth in square feet

$\mathbf{T_c}$ = top width of critical flow in feet.

For non-uniform cross sections, a rating curve of critical depth versus discharge shall be constructed.

Once the discharge **Q**, area **A**, and depth **d** are determined, the slope necessary to produce these conditions in a channel can be computed from Manning's Equation.

3. Design of Open Channels

Traditional Designs

The criteria outlined in this section are intended to guide the development of traditional designed/constructed open channels. Roadside ditches shall be designed as open channels per the Guidelines in Paragraph D4 of this Section. Alternate channel designs will be considered by the City Engineer provided they are shown to meet the intent of these Guidelines.

Natural Designs Encouraged

Designs intended to replicate the characteristics of natural streams are encouraged but must be shown to satisfy the essential purposes of the provisions of this paragraph. Example features that might be considered for such designs are among those outlined in Appendix E.

a. Physical Considerations

(1). Cross-Section Geometry

The minimum standards acceptable for use in traditional lined channel design is in the Cibolo Design & Construction Manual. The maximum side slope shall be four horizontal to one vertical (4:1).

(2). Minimum and Maximum Grades

The minimum longitudinal slope shall be 0.006 foot per foot (0.6 percent) for earthen or vegetative lined channels to prevent formation of standing water. The maximum allowable grade shall be a function of allowable flow velocity as related to channel lining materials stipulated in Table C-11 (Appendix C). If the proposed maximum grade will exceed 70 percent of the calculated critical slope values for the required range of design flows, special channel linings and energy dissipation features must be used to compensate for the high velocities and hydraulic jumps associated with supercritical flow. Designs for supercritical flow are limited to straight sections having a minimum grade that is at least 130 percent of the critical slope values calculated for the required range of design flows.

(3). Bends and Horizontal Curves

The maximum allowable deflection angle for bends in designed channels shall be 45 degrees. The outside of horizontal curves shall provide additional channel bank height and surface treatment as necessary to fully contain the design flow and prevent erosion and overtopping.

(4). Erosion Protection Measures

Measures required for protection of earthen channels are described in Section VII of these Guidelines (see Paragraph C3).

(5). Berms

If earthen berms are proposed as permanent features for stormwater management they shall meet a structural compaction of 98 percent Standard Proctor. Berm side slopes shall be a maximum of three horizontal to one vertical (3:1) if to be privately maintained and four horizontal to one vertical (4:1) if to be publicly maintained. As a function of height, berms shall have a minimum top width as follows:

- Height 2 feet or less 3 feet top width
- Height between 2 and 6 feet 5 feet top width
- Height exceeding 6 feet 10 feet top width

b. Flow Considerations

(1). Design Flows

a). Channel capacity shall be determined to accommodate the discharge from a 25-year storm assuming buildout conditions for all of the Project Area of a land development project that can be foreseen to discharge to the channel, plus the 100-year storm flow from existing conditions on all other land areas served by the channel.

b). Channels shall be designed to flow supercritical for the range of discharges resulting from the 5-year, 10-year, 25-year, and 100-year design storms on the Design Drainage Area.

c). When a low-flow flume is to be constructed in an open channel, an invert section of concrete or other materials acceptable to the City Engineer must be designed to carry 33 percent of the peak design discharge of a 5-year storm for the channel as stipulated in the previous sub-paragraph.

(2). Velocity Limitations

a). Velocity of flow shall not be less than two and one half (2.5) feet per second for the 25 year design storm.

b). Maximum velocities for the design flow shall be less than the values given in Table C-11 in Appendix C for the type of surface treatment(s) specified.

(3). Freeboard Requirements

Channels shall be designed with a minimum freeboard equal to one foot above the 25 year design depth of flow.

c. Outfall Junctures

Junctures Important Where part of a storm drainage system discharges into another part of the system, on-going long-term maintenance difficulties can result, particularly where the receiving facility is an open channel. The complexity and importance of these junctures warrants careful design attention.

Juncture Categories Junctures can be grouped into three categories: discharge from an underground storm sewer conduit into the secondary or primary drainage system; discharge of an open flume into the secondary or primary drainage system; and the confluence of two channels (secondary/secondary or secondary/primary).

Public System The following guidelines apply to points of discharge into the public stormwater conveyance system, whether from a private or public drainage facility.

(1). Storm Sewer Outfall Points

Acute Connections Where storm sewer lines are to discharge directly into culverts or channels they must do so at an acute angle (preferably not exceeding 45 degrees) so that flow is generally in the same direction as the flow of the receiving facility. Where discharge is into a culvert, the connection should match the soffit elevation of the two facilities as closely as practical. Connection details and grouting shall be in conformance with the City of Cibolo Design and Construction Manual.

Match Inverts Where discharge is directly into a designed or natural watercourse, the discharge invert elevation should match that of the receiving facility as closely as practical. Alternatively, special channel treatment designs may be proposed so that the outfall discharge will not inhibit or obstruct flow in the receiving channel. In either case, the design must work to manage the velocity of the outfall discharge to prevent scour of the bottom or sides of the receiving channel.

(2). Flume Outfall Points

No Erosion, Scour Flumes that convey stormflow into a natural or designed watercourse shall be designed to prevent storm flow from interfering with the integrity of the bottom or sides of the receiving facility. This will necessarily involve managing discharge velocity to avoid scour, as well as possible treatment of portions of the receiving water course. No such connection shall inhibit or obstruct conveyance of the design storm flow of the receiving water course.

(3). Points of Channel Confluence

Control Turbulence Channel confluences should be at 45 degrees or less, and the design should bring flows together as nearly as possible at the same velocity in order to minimize turbulence. The design must include treatments to ensure adequate erosion control consistent with provisions in Section VII of these Guidelines.

4. Roadside Ditches

Where the use of roadside ditches is approved by the City Engineer, the design shall be governed by provisions for open channel flow as set out in the forgoing paragraphs of this Section, unless superseded by higher or more explicit standards as outlined below.

a. Hydraulic Design of Ditches

(1). Ditches must completely contain the flow from the design 25-year storm with a water surface elevation six (6) inches below the top of the ditch.

(2). The maximum 25 year design depth of flow shall be limited to three (3) feet.

b. Ditch Geometry

(1). Culverts must be at least 18 inches in diameter.

(2). The top of the ditch bank must be separated laterally from the roadway shoulder (edge of base course) by at least two (2) feet.

(3). Ditch sections shall have a minimum depth of one and one half (1.5) feet.

(4). Side slopes shall be no steeper than four horizontally to one vertical (4:1).

c. Ditch Construction

(1). Culverts and grading shall be constructed in compliance with City of Cibolo Design and Construction Manual specifications.

(2). All ditches must be completely vegetated in accordance with City of Cibolo Design and Construction Manual specifications.

(3). All computations and design drawings shall demonstrate satisfaction of design provisions of these Guidelines.

5. Modification of Natural Watercourses

a. FEMA and “Non-FEMA” Systems

Both the Primary and Secondary Systems include natural watercourses of various sizes and capacities. The great majority of these watercourses form the FEMA-designated Floodplains as defined in paragraph G of this Section. Most of the remaining natural watercourses are generally upstream extensions of those forming the FEMA-designated system. For purposes of these Guidelines natural watercourses shall be considered to be in one of two categories: as part of the Named Regulatory Watercourses defined in Section II (the “FEMA-Designated Flood Plain System”), or as “Non-FEMA” watercourses.

b. FEMA-Designated Flood Plain System

Watercourses making up the FEMA-Designated Flood Plain System must be in compliance with the requirements of paragraph G of this section, in addition to provisions of this paragraph (D-5) and its subparagraphs.

c. Principles

(1). Modifications shall be defined as physical changes in a watercourse's vertical and/or horizontal alignment, cross-section geometry, surface characteristics, or over-bank areas. Movement or addition of earthen materials, grubbing, and wholesale removal of vegetation is considered modification activity, but trimming of vegetation is considered maintenance and is not governed by these Guidelines.

(2). At a minimum, all modifications to natural watercourses shall meet the requirements governing design or improvement of open channels stipulated elsewhere in these Guidelines.

(3). Changes to natural watercourses must be consistent with an approved master plan for modification of a complete reach of the Primary System if such a master plan exists. If no plan exists, one may be required at the discretion of the City Engineer. Changes to short parts of a natural watercourse must demonstrate compatibility with similar modifications along the length of that reach, whether existing or planned.

(4). On any site that is a single platted lot, minor encroachments, consisting of fill and earthwork changes in existing defined floodway fringe areas may be allowed at the discretion of the City Engineer. Any encroachments shall meet all requirements listed in the following sub-paragraphs.

d. Determination of Floodway and Floodplain Areas

(1). For streams forming the primary drainage system, a comprehensive hydraulic model, has been adopted. This study shall be used as the principal source defining floodway and floodplain areas for streams and channels making up the primary system.

(2). Along streams and channels lacking an existing study, floodway and floodplain areas shall be determined by extending the adopted Flood Study using the standard step procedure. Where new flood discharges must be determined, they shall be computed using the methods outlined in Section V of these Guidelines.

(3). Land development projects proposing to use land filling or berms or structural features to raise existing floodplain areas above flood levels are considered encroachments into floodplain areas. Because this will raise the base flood elevations (BFE) in the vicinity of the proposed work the extent of encroachment must be limited so that the BFE is not raised by more than one foot. These geographic limits will define the resulting “floodway” for that Watercourse, or tributary thereof. This effect is illustrated in Figure C-3, Appendix C.

(4). The floodway shall be determined using an encroachment method based on proportionate conveyance reduction (as a function of hydraulic cross sectional areas) from both sides of the channel over-bank. However, the limits of encroachment shall not extend into the designated channel area. The engineering studies necessary to identify “floodways” rests with the owner/developer (or the applicant) of the proposed project at the discretion of the City Engineer or his/her designee.

e. Design Considerations

(1). Analysis for System Impacts

Modified Channels

When existing channels are straightened, improved in cross-section, and/or lined, their hydraulic efficiency increases. Such action results in reduced travel times and reduced times of concentration. It can also result in loss of over bank storage capacity. These factors cause higher flood discharges and higher flood elevations downstream of the area of improvement. Any changes to channels within the Primary System shall be accompanied by a revised analysis of the hydrologic model (both current condition and ultimate condition) of the adopted Flood Study. The changes will be reflected in the routing reaches and lag factors for affected channel reaches and s.

Downstream Effects

Downstream impacts shall be reviewed to prevent damage to existing property and structures. Key items shall include the effect of higher discharges at bridges and culverts, and the changes in flood elevations. Channel improvements shall not cause increases in flood discharges that will exceed the capacity of downstream crossing structures, and shall not raise ultimate 100-year flood elevations.

(2). Transition Sections

Smooth Transitions

Modification of any channel section shall include designs to affect smooth transitions with the existing channel features, both upstream and downstream. These transitions should be gradual to prevent the formation of excessive energy losses and turbulence, or the creation of inappropriate velocities in upstream or downstream sections of the channel. Any proposals for abrupt changes in section, profile, or alignment must be accompanied by engineering studies demonstrating that planned energy dissipation measures will preserve the long term integrity of channel elements. Energy dissipation measures must be acceptable to the City Engineer.

E. Detention Facilities

1. Principles

Controlled Discharge

The purpose of a detention facility is to store excess stormwater runoff and discharge it at a predetermined controlled rate. Typically, this is done so that discharge rates from a development site will be limited to those that existed prior to any land development activities. This is accomplished for a range of design storms.

Facility Types

As a function of how they are designed to operate, detention facilities can be grouped into three categories. One type is effectively a permanent pond. That is, it retains a significant water pool on a year-round basis, but acts to detain stormflow, metering water release until some predetermined pool level is reached. This might be termed a “pool-type” (retention) facility. Another type might be termed a “wetland-type” facility. This type retains storm flow and meters its release, but is not intended to drain fully dry. Rather, an aquatic ecosystem is specifically designed into part or all of the facility so that it is sustained by the storm flow that passes through the facility. The third type is designed to drain fully dry between storm events, a “dry-type” facility.

Detention Philosophy

These Guidelines are largely oriented toward development of “dry-type” facilities. However, where topographic, water, and other physical characteristics make it feasible to design viable “wetland-type” facilities, they are encouraged. Successful “wetland-type” or “pool-type” facilities can be difficult to establish and are highly dependent on an expert multi-discipline design team for their success. Use of a “wetland-type” or “pool-type” facility will be considered a special design, and must be approved by the City Engineer on a case-by-case basis. The City Engineer must be informed early during the planning of a project. In addition, the design must be handled by qualified professionals, experienced in establishing self-sustaining wetland environments. The stormwater detention function shall not be compromised by such special designs.

Drained Areas

Detention facilities may be site-specific, or may be designed for a specific land development project comprised of multiple lots, streets, utilities, and other infrastructure elements. In any case, their primary purpose is to protect immediate downstream properties and drainage system from excessive stormflow. One detention facility, or a system of facilities, may be necessary to meet stormwater management objectives for an entire Project Area. A site-specific example would be using a detention facility in a large parking area to avoid overwhelming adjacent streets and storm sewers of the secondary system. Common methods include use of depressions in parking lots and/or landscaped areas that drain dry between rainfall events.

Regional Detention Detention facilities also may be regional in scope, receiving stormwater from many land development Project Areas and/or sites. In such situations a limiting capacity is often that of the drainage system that traverses an existing developed area.

Multi-Purpose Areas A regional facility requires a large land area for the required storage and, thus, is usually designed for multiple uses compatible with its stormwater purpose. For best results, these are permanent storage ("pool-type") facilities designed to hold water between rainfall events, and may be combined with green-space and recreation areas.

"Regional" Limited Detention facilities will only be considered "Regional" at the discretion of the City Engineer.

2. Design Parameters

a. Design Storm

Secondary System Any detention facilities to be located in the Secondary Drainage System that are site-specific, or will serve a specific development project, shall use a maximum design storm based on specific detention requirements stipulated in these Guidelines. The following sequence of design storms shall be used until the maximum design storm is reached: 10-year, 25-year, and 100-year. Full consideration must be given to the receiving facilities of the secondary system relative to performance standards and Conveyance Pathway requirements. In addition, the 100-year design storm shall be evaluated to check emergency overflow requirements of the detention facility and the effects of resulting flows on downstream drainage systems.

Primary System Where detention facilities are required to be located in the primary drainage system, either on-line (astride the main channel) or as an adjacent flood relief feature, they shall use a maximum design storm having an average return period of 100 years or greater as determined by the City Engineer.

b. Delineation of Drainage Area

Each detention facility shall serve a Design Drainage Area that contributes (or will contribute) runoff to the facility. The Design Drainage Area and the runoff computations shall be determined for existing pre-development conditions and for expected post-development conditions.

c. Pre-development and Post-development Hydrographs

A pre-development hydrograph representing the Design Drainage Area and land cover conditions existing prior to the proposed development shall be determined. Likewise, a post-development hydrograph shall be determined representing the Design Drainage Area and land cover conditions proposed to exist after buildout of the Project Area that contributes runoff to the detention facility.

Hydrographs shall be determined using the appropriate methods from Section IV (Hydrology) of these Guidelines.

d. Determination of Storage Volume

Pre/Post Flows

Storage volume shall be adequate to ensure that the peak discharges from the detention facility determined via the post-development hydrographs will be limited to values equal to, or less than, the peak discharges determined by the pre-development hydrographs for the design storms.

Existing Storage

Any land features, such as low areas or ponds, having the effect of storing or detaining stormwater during pre-development conditions shall not be ignored in determining the required post-development storage volume. If such features are to be altered or eliminated, then the required storage volume must be increased to account for their pre-development detention characteristics. The existence and effects of such features shall be disclosed during the design review process.

Storage Routing

All detention facilities shall have the necessary storage volume determined from storage routing analysis procedures.

e. Storage Routing Analysis

The basis of this method is the continuity equation modified to yield the following:

Where:

I = the inflow over time period t,

O = the outflow over time period t,

S = the storage volume,

dt = the designated time period, and

subscripts 1 and **2** represent the beginning and end of time period respectively.

$$(I_1 + I_2) + \left(\frac{2S_1}{dt} - O_1 \right) = \left(\frac{2S_2}{dt} + O_2 \right)$$

The use of this procedure is based the following assumptions:

- The inflow hydrograph is known.
- The starting conditions of storage volume and outflow are known at the beginning of the routing.
- The discharge rate at the outlet structure(s) is only a function of the head available.
- The relationship between depth and storage are known.
- The time period "**dt**" shall be taken as less than or equal to 1/5 $\mathbf{t_c}$ (time of concentration).

f. Outlet Structures

(1). Design of outlet structures shall consider the conditions for all required design storms. The structure shall limit the peak discharge to be equal to, or less than, the peak discharge that existed under pre-development conditions for all design storms.

(2). Except for facilities designed to have a permanent storage component, outlet structures shall be designed to allow the facility to be drained dry by gravity.

(3). An emergency overflow outlet shall be provided with a capacity to carry the peak discharge from a 100-year frequency storm for buildout conditions over the entire Design Drainage Area. This discharge must be limited and directed in a manner that will: prevent damage to adjacent properties or public infrastructure; avoid damaging the structural integrity of any element of the detention facility; and present no hazardous conditions. In addition, the discharge shall be evaluated for its effect on the downstream receiving drainage system, and shall not exceed its capacity to control and contain the storm discharge assuming ultimate conditions.

(4). Analysis and design of outlet works shall use the methods promulgated by these Guidelines, namely those dealing with drainage inlets, drainage conduit, open channel flow, and culverts. In addition the City of Cibolo Design and Construction Manual shall apply.

3. Physical Characteristics For Dry-Type Facilities

a. Side and Bottom Slopes

(1). Side slopes shall not exceed 4:1 for vegetative cover and 2:1 for non-vegetative cover.

(2). Bottom slopes must be a minimum of 5 percent (5%) for a vegetative cover and 0.5% for a flume section or steeper and directed to the low flow outlet.

(3). A low-flow invert section of concrete or other materials acceptable to the City Engineer shall be provided for all facilities proposed to have a bottom with vegetative cover. To minimize the need for these sections, designs are encouraged to locate the inflow and outflow points as close to each other as practical.

b. Emergency Overflow Requirement

(1). All detention facilities shall be fitted with an emergency overflow feature that discharges into a recognized drainage facility acceptable to the City Engineer.

(2). The geometry of an emergency overflow structure shall be that of a rectangular or trapezoidal weir.

(3). The surface treatment of the structure and its discharge path to a recognized drainage facility shall give due regard to maintenance. Velocities shall be limited to be consistent with the proposed surface treatments to prevent erosion, prevent undercutting of structural components, and avoid other maintenance difficulties.

(4) The elevation of the weir crest shall not be less than the water surface elevation resulting from the design 100-year storm, assuming a fully operating discharge structure. See diagram presented in Figure C-4 in Appendix C.

(5). The entire perimeter of the facility shall have at least one half (0.5) foot of freeboard above the water surface elevation generated by the 100-year storm assuming buildout conditions of the Design Drainage Area, a completely clogged discharge structure, and a fully functioning spillway.

c. Storage Depth

In parking areas the maximum design storage depth, based on site buildout conditions, shall not exceed six (6) inches.

d. Retention (Permanent Storage) Facilities

All facilities located astride natural streams or water courses that are designed with a permanent storage component shall meet all design and construction criteria for dams and reservoirs as required by the Texas Commission on Environmental Quality (TCEQ).

e. Allowance For Sedimentation

The design storage capacity of detention facilities shall be increased by ten percent (10%) to allow for sedimentation.

E. Culverts and Bridges

SECTION V

HYDRAULIC DESIGN

1. Principles

Transportation Purpose The purpose of a culvert or bridge is to allow a transportation facility to cross a drainage way. Consequently, its primary function is to satisfy transportation purposes. Designs to accomplish this end necessarily involve satisfying both hydrologic and transportation parameters.

Design Objectives Hydrologic parameters are established to achieve important design objectives: safety of transportation users; safety of surrounding properties; long term integrity of constructed facilities; minimum maintenance costs; and integrity of the natural environment.

Parameters Vary Not all parameters are universally applicable to drainage way crossings. Because transportation facilities (roadways) vary in their function and importance, related hydrologic parameters are varied accordingly. Conversely, parameters relating to the integrity and maintenance of constructed facilities, and those relating to potential flooding of adjacent properties cannot vary.

2. General Parameters

100-Year Discharge The design storm discharge shall be based on the ultimate development conditions that are projected to exist in the Watershed or served by the watercourse to be crossed. In addition to satisfying parameters for passing the design discharge, the 100-year storm flow must be accommodated. Arterial and major collector roadways are not to be toped by flow from the 100-year design storm. Certain minor roadways may be toped according to criteria set out in Paragraph F3-c below.

Minimize Erosion Structures shall include design features that can receive the discharge of street or storm drain flow in a manner that will prevent erosion or scour of adjacent embankments or the floor or walls of the channel. Typically, a concrete apron or other suitable surfacing shall be provided to receive the discharge.

Flood Hazard Areas Structures within established areas of special flood hazard as defined by the flood plain management ordinance(s) of the City shall meet all the requirements for those areas as a minimum. These Guidelines supersede provisions for such areas only to the extent that more stringent requirements are promulgated.

3. Design Limitations and Performance Criteria

a. Determination of Design Discharges

(1). For structures over Named Regulatory Watercourses or their direct tributaries, the design discharges shall be determined from adopted flood studies of the City per Section II of these Guidelines.

(2). For structures over watercourses making up the secondary system, the design discharges shall be determined using the appropriate methods outlined in Section IV of these Guidelines.

b. Maximum Operating Headwater

(1). For all discharges up to and including the 100-year frequency storm culverts shall be designed to limit upstream headwater to elevations that will not endanger their structural integrity or cause flooding to adjacent structures or properties.

(2). At bridge crossings the water surface elevation of the 100-year storm flow shall not be higher than one (1.0) foot below the lowest bridge support stringers.

(3). For culvert crossings the upstream headwater elevation for the design discharge shall be at least one (1.0) foot below the lowest top of curb at the crossing.

c. Allowable Over-Road Flow

Over Minor Roads

Where a roadway classified as a local street or minor collector will be topped by flow from a 100-year frequency storm due to allowable lesser design storms for a culvert, the excessive storm flow may be conveyed over the roadway provided the following criteria are met.

(1). Roadway and storm drainage features must be designed so that all over-road storm flow is conveyed across the road and routed to the downstream watercourse without endangering adjacent properties or structures.

(2). The maximum depth of over-road flow shall be two (2.0) feet, measured from the roadway crown at the lowest point in the roadway profile.

(3). Considered together, the velocity and depth of over-road flow provide an indication of the potential detriment to the structural integrity of the roadway. Therefore, the product of the velocity of the overflow discharge (in feet per second) and the maximum depth of flow (in feet) as described in the foregoing paragraph shall be less than six (6), a dimensionless number. The overflow velocity shall be determined from the continuity equation as follows.

$$V = \frac{Q_{over}}{A}$$

where:

V = velocity in the overflow discharge, feet per second.

Q_{over} = maximum discharge over roadway, cubic feet per second.

A = area of the overflow section described by the headwater elevation and roadway profile at the crown.

d. Maximum Discharge Velocities

The velocity of discharge through a structure shall be limited based on channel conditions immediately downstream of the structure. Reference is made to Table C-11 in Appendix C. For discharges from the five-year design storm, downstream conditions will be evaluated to the point where normal flow characteristics are re-established in the receiving channel, but not less than a distance that is four (4) times the difference between the width of the downstream flow and the width of the structure opening. This does not apply for discharges from less frequent storms.

4. Physical Configuration

a. Alignment Criteria

Match Flow Lines

Bridges and culverts beneath roadways should provide flow lines that match, as closely as possible, the alignment of the watercourse they are to serve. At the same time, it is desirable for watercourses to cross roadways in a perpendicular manner. Where both of these design objectives cannot be reasonably satisfied, the amount of skew in crossing a roadway should be minimized. In addition, the hydraulic demands resulting from introducing any artificial turns in a watercourse must be fully accommodated by the design.

Driveway Culverts

Where driveways must cross roadside ditches, culverts shall be placed in public right-of-way, generally parallel to the street, and aligned with the flow line of the ditch.

Straight Structures

Changes in bridge or culvert alignment shall not occur within the right-of-way of the roadways they cross.

b. Right-of-Way / Easements

ROW At Roadways

At roadway crossings right of way must be provided to fully contain all bridge and culvert features, including headwalls, end-walls, wing-walls, and any support structures. This can be in any combination of right-of-way for the roadway and/or the watercourse.

100-Year Easements

Where culverts are designed to convey flow less than that generated by the 100-year design storm, areas inundated by backwater conditions shall be wholly contained in right-of-way or drainage easements.

Pass 100-Year

Bridges are to be designed to pass the flow from the design 100-year storm and, therefore, are not to create a design backwater condition requiring easements or right-of-way. If storm flow exceeding the 100-year design is to be routed around a bridge opening and over the roadway approaches, right-of-way must be provided for the path of the routed flow.

c. Culvert Ends

The following guidelines shall be used in designing culvert end treatments. Figure C-5 (Appendix C) shows a schematic diagram illustrating terms commonly used to describe a typical culvert structure.

(1) Concrete headwalls and end-walls shall be provided to be functionally monolithic with the culvert conduit and must generally be parallel with the alignment of the crossing roadway. Related wing walls shall generally be oriented according to the flow characteristics of the crossing watercourse. In no case shall headwalls or wing walls restrict the clear opening of the structure.

(2) Flared wing-walls shall be used where both of the following conditions apply:

- Approach velocities exceed six (6) feet per second for the design discharge
- The approach channel is irregular and not well defined.

(3) Wing-walls parallel to the flow line of a watercourse may be used where all of the following conditions are met:

- Approach velocities are less than six (6) feet per second for the design discharge, and
- The channel is well defined and regular in cross section, and
- Downstream channel surface protection is not necessary.

(4) The maximum side slopes for all grading in the vicinity of culvert headwalls shall be six horizontal to one vertical (6:1), unless 4:1 or flatter is approved via a design exception approved by the City Engineer.

5. Bridge and Culvert Hydraulic Design

a. Analysis Methodology

Bridge Hydraulics

The following items shall be addressed as part of the engineering design and analysis of crossing structures. Bridges shall be analyzed for hydraulic conditions using the HEC-RAS Water Surface Profiles computer program applied using the guidelines and recommendations of the U.S. Army Corps of Engineers. Unless other parameters can be substantiated to the satisfaction of the City Engineer, the following nine shall apply:

- A combination of TP40 and Hydro 35 Precipitation Data as provide in Table C-6, Appendix C.
- 10, 25, and 100 year rainfall runs.
- Lag Times for the unit hydrograph should be computed using the NRCS (SCS) lag equation.

- Rational Formula should be used for the peak Q from Design Drainage Area less than 50 acres in size.
- Balanced triangular hydrograph for the PH record in HEC-1 should be used for draining between 50 and 200 acres, and lag times less than 30 minutes.
- NRCS (SCS) Type III, 24-hour duration storm should be used for drainage s larger than 200 acres or lag times exceeding 30 minutes.
- Modified-PULS for Channel Routings and PULS may be used for steep slopes.
- Losses should be computed using the NRCS (SCS) curve number method.
- The NRCS (SCS) unit hydrograph technique is encouraged where no data is available to estimate other parameters.

Culvert Hydraulics

Culverts may be analyzed using the same method as for bridges. Additionally, they may be analyzed using accepted charts and nomographs for the type of structure and material proposed for use. TxDOT's Hydraulic Manual contains a complete treatment of culvert analysis and design, including nomographs. The latest version of TxDOT's Hydraulic Manual shall be considered the standard for analysis of culverts by these Guidelines.

b. Culvert Operations

The rate of flow through a culvert barrel is limited by several direct factors such as slope, length, and surface roughness. Where conditions at the culvert entrance (inlet) prevent optimum flow based solely on these factors, the culvert is considered to operate under "inlet control". When the flow permitted through the barrel is less than the flow allowed at the upstream entrance, the culvert is considered to operate under "outlet control" (sometimes referred to as "barrel control"). For each design discharge the type of control shall be determined.

c. Headwater and Tail Water Elevations

(1). Tail water elevations shall be determined using one of the methods described in the portion of this Section guiding open channel design (see paragraph D2-a).

(2). Headwater elevations shall be determined by adding the total head losses through the structure to the tail water elevation, for the given discharge.

d. Head Losses

The total head losses, **H**, on a structure is the sum of all losses due to exit, friction, and entrance conditions for the given discharge.

(1). Entrance losses are caused by the narrowing of flow from the normal channel width to the structure opening (predominant for bridges), or to the shape or condition of the actual inlet or opening (predominant for culverts). Channel losses of this type must be computed using a standard step procedure as outlined in the part of this Section dealing with open channels. Entry losses shall be computed using the following equation:

$$H_e = k_e \left[\frac{V_2^{\,2}}{2g} - \frac{V_1^{\,2}}{2g} \right]$$

where:

H_e = entrance head loss, feet

V_2 = velocity of flow in culvert, feet per second

V_1 = velocity of flow in approach channel, feet per second

g = 32.2 feet per second per second

k_e= entry loss coefficient from Table C-12, Appendix C.

(2). Exit losses are caused by the expansion of flow from the structure opening to the normal downstream channel width. The same equation for entrance losses applies to those for exit losses except **k_e** may be taken as **1.0** and **V_1** shall be defined as the velocity of flow in the downstream receiving channel after full expansion.

(3). Friction losses are those that occur within the structure itself. These can range from open channel flow losses, and pressure flow losses, to losses caused by physical obstructions within the structure (bridge piers for example). All friction losses shall be accounted for in the analysis and design of crossing structures.

e. Erosion and Scour Protection

(1). All culverts determined to be functioning under inlet control for the design discharge shall have an energy dissipation structure at the outlet of the culvert or meet the requirements of Paragraph 5e-(2) below.

(2). The velocity of the design stormflow in the structure shall not exceed the requirements for the downstream channel condition stipulated in Table C-11, Appendix C.

G. Floodplains

1. Principles

Floodplain Definition

A "floodplain" is generally land areas along and near a waterway that are inundated during large and relatively infrequent storm events. The runoff from smaller, more frequent storm events is generally contained within the main channel of the waterway and has little to no effect on adjacent "over-bank" land areas.

Width Varies

Fundamentally, every watercourse has attendant floodplain areas that can be situated along one or both sides of the main channel, depending on topographic features. Along smaller streams or channels there may be little distinguishable difference between the main flow area and the floodplain. However, on larger streams or channels floodplain areas may be very broad and shallow, and may provide for very little conveyance of stormwater.

Public Policy

Due to rather infrequent flooding of over-bank areas and other factors, property owners often have interest in establishing urban land development in flood-prone areas, particularly in broad shallow floodplain areas. Consequently, public policy, by all levels of government, has established mechanisms designed to mitigate the negative effects of using floodplain areas. One of the purposes of these Guidelines is to facilitate those policies in the Cibolo area.

2. Identification of Floodplains

Identified Floodplains

Floodplains are principally associated with the primary drainage system. The primary system and its tributaries make up the Named Regulatory Watercourses listed in Table B-1 (Appendix B) of these Guidelines. The over-bank areas of these waterways are considered to be the "identified" floodplains, even though the specific geographic limits of some reaches of each watercourse system are not dimensionally defined in hydrologic and/or topographic terms.

Floodplain Limits

As land development occurs along the Watercourses identified in Table B-1 (Appendix B) of these Guidelines, and along upstream extensions thereof, it will be necessary to fully define the geographic limits of the attendant floodplains. This will allow application of these Guidelines to those areas in a precise manner, thus defining hydraulic engineering needs, land development parameters, and private/public interests.

3. Regulations

FEMA Flood Studies

A series of several FEMA-approved hydrologic studies have been conducted to determine the floodplain areas along the majority of the reaches of the Named Regulatory Watercourses listed in Table B-1 (Appendix B). These are the FEMA-designated watercourses in the Cibolo area. Taken together, the flood studies conducted for these Watercourses represent the "Flood Study" of the City.

Areas Not Defined In some instances the floodplain areas along upper reaches of a Watercourse are undefined even though the floodplain clearly extends beyond areas shown on FEMA maps. In other instances floodplain areas may be ill-defined due to topographic or other constraints.

Define Limits Land development or building projects proposed on properties astride of, or adjacent to, the Watercourses listed in Table B-1 (Appendix B) may require flood studies in order to precisely identify the elevation and geographic limits of potential floods, and thus the mitigation measures necessary for the project(s). If a proposed development will involve more than 50 lots or five (5) acres at buildout, a comprehensive flood study may be required at the discretion of the City Engineer.

Special Use Areas In land areas set aside for parks or other recreational or green space uses, or proposed for such uses, special regulations by the City may require adjustments in how these Guidelines are applied. Any deviation from provisions of these Guidelines in such areas will be at the discretion of the City Engineer or his/her designee.

4. Procedures

If Study Needed When a comprehensive flood study is needed for a land development or building project, a number of procedures are required. The hydrologic analyses criteria and methods stipulated in Section IV (Hydrology) of these Guidelines and those stipulated in Paragraph D5 of this Section will apply. For minor streams or channels that are tributaries to the Named Regulatory Watercourses, existing and ultimate flood elevations shall be established by extending the adopted Flood Study as described in foregoing Paragraph D5-d.

Plot Limits Water surface elevations based on the configuration and limitations of the existing channel shall be determined for the ultimate development conditions planned by the City for the Watershed involved. The resulting geographic limits of projected flooding will be plotted by the engineer conducting the study.

Channel Changes When existing channels are straightened, improved in cross-section and/or lined, existing floodplain and floodway areas are likely to be altered. Redefinition shall follow the methodology for floodway determination outlined in Paragraph G2 of this Section.

Limited Effects Proposed changes in channel section or alignment shall not increase the existing or ultimate flood elevations (established by the adopted Flood Study) within, or upstream or downstream of, the area of modification, more than allowed by these Guidelines. Any changes shall be made part of the adopted Flood Study and submitted to the required authorities for approval prior to construction work in floodway or floodplain areas.

5. Code of Ordinance Compliance Required

Chapter 30 Floods Plans shall comply with City of Cibolo Code of Ordinances, Chapter 30, “Floods”.

Section VI
Appendices

A. Authority

B. Region's Watersheds

C. Computational Information

D. Technical Design Summary

E. Best Practice

F. Glossary

G. General References

Appendix A
Authority

SECTION VI

APPENDIX A – AUTHORITY

These Uniform Stormwater Design Guidelines regulate the design philosophies and criteria that are to be used in assessing the need for and design of stormwater management facilities planned and engineered for land development projects within the jurisdiction of the City of Cibolo. Important purposes are: 1) to offer the citizens of the city a single set of requirements that clearly define what must be done to satisfy the broad policies of the city, and, 2) to achieve greater uniformity of resulting stormwater facilities. To those ends, these Guidelines work to implement stormwater management ordinances adopted respectively by the City of Cibolo for use in their respective jurisdiction.

These Guidelines derive their authority from the stormwater management ordinances, reports, studies and floodplain management ordinances adopted from time to time by the City Council of Cibolo.

Appendix B
Region's Watersheds

SECTION VI

Regions Watersheds

(RESERVED)

Watershed Name	Channel Reach		Detention For Flood Control
	From	To	

APPENDIX B – REGION'S WATERSHEDS

HYPOTHETICAL MAP (To Be Replaced By Cibolo Regional Watershed Map)

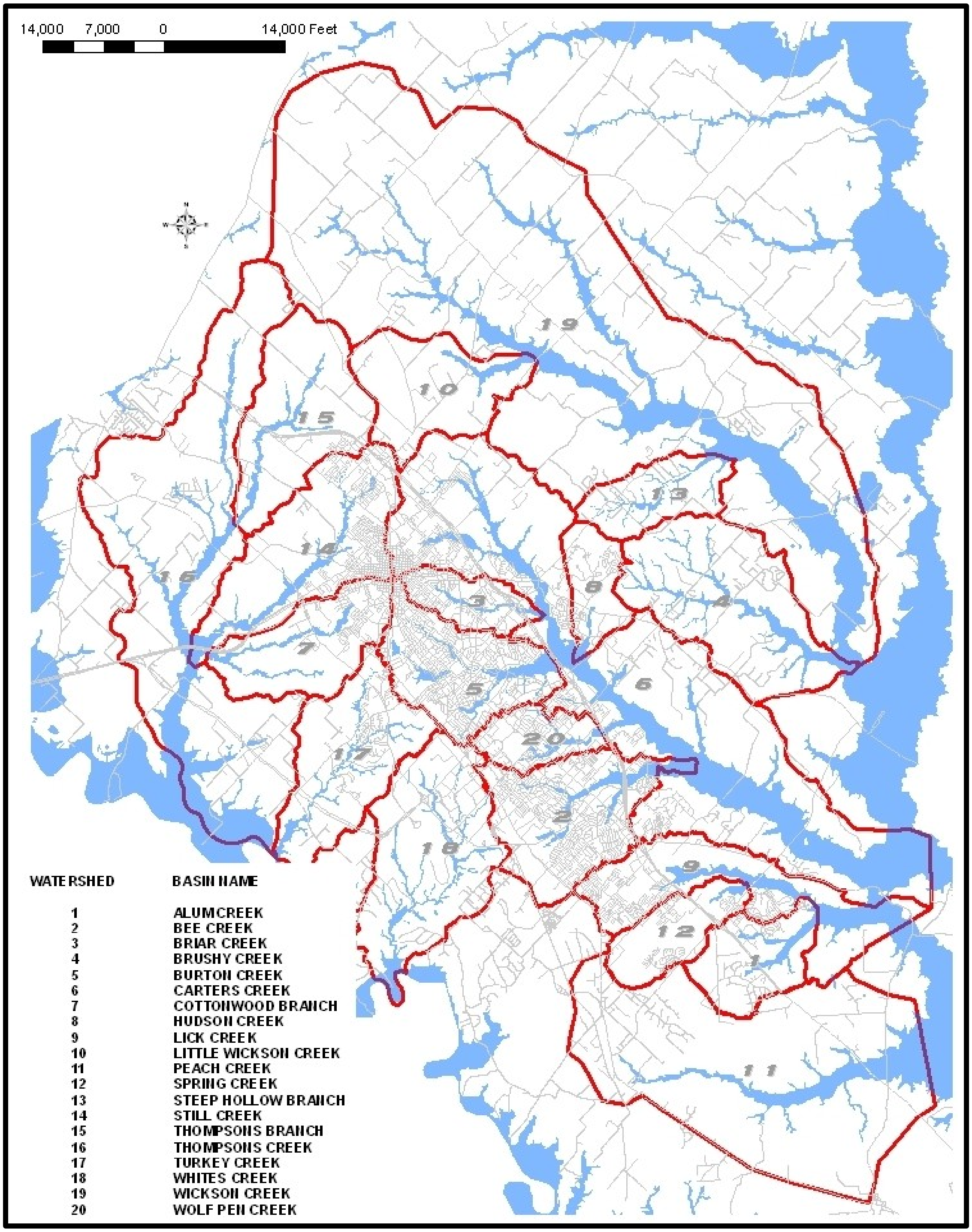


Figure B-1: Watersheds of Cibolo Region

APPENDIX B – REGION'S WATERSHEDS

HYPOTHICAL MAP (To Be Replaced By Cibolo Regional Watershed Maps)

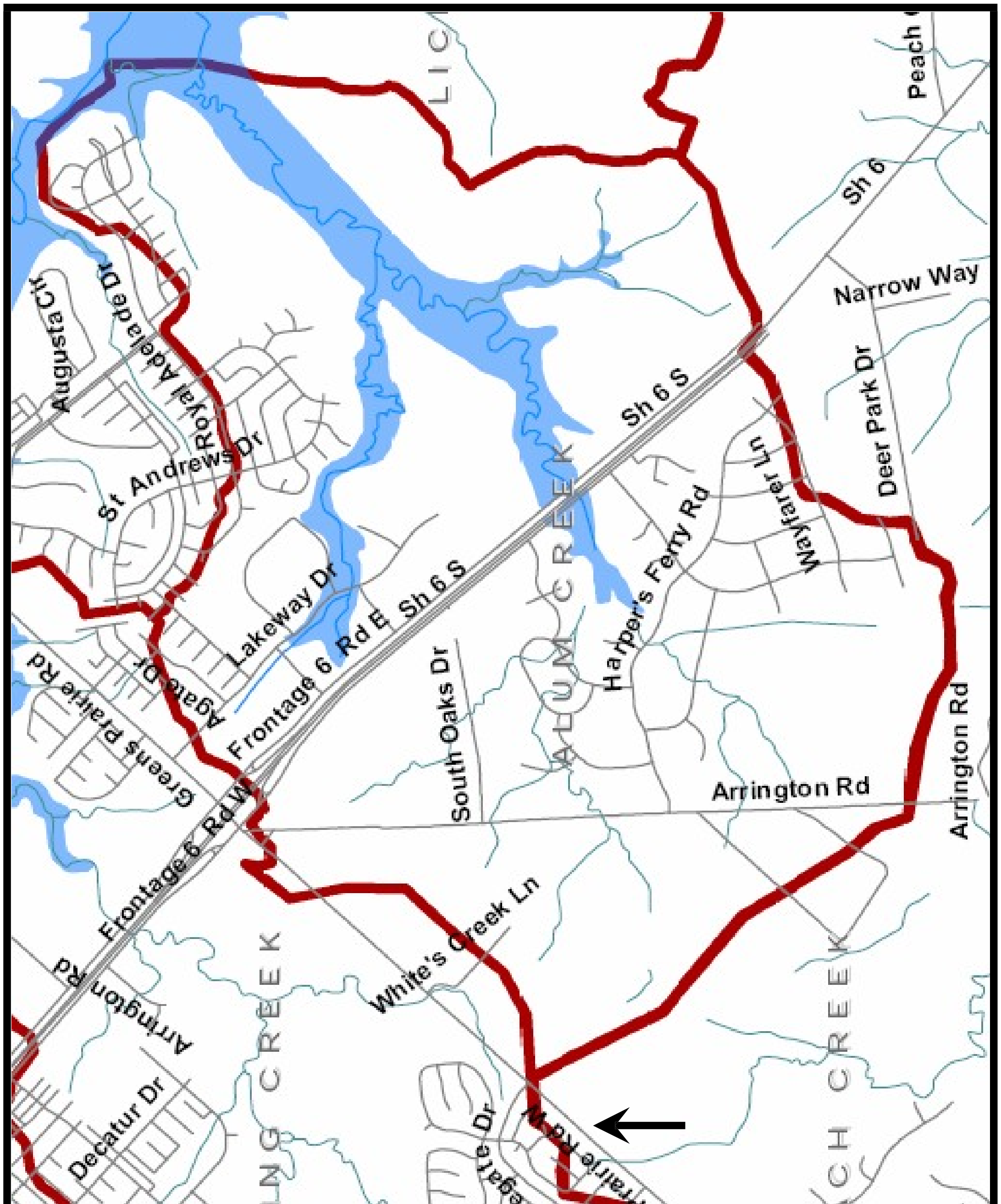


Figure B-2: Town Creek East Tributary 3 Watershed Area

Appendix C Computational Information

Hydrology Computational Information

This portion of Appendix C provides tables and figures in support of the methodologies stipulated in Section V of these Guidelines dealing with the application of hydrologic principles. It includes the following Tables and Figures:

- Table C-1: Equations for Calculating Rainfall Intensities
- Table C-2: Runoff Coefficients (C) by Land Use Type
- Table C-3: Runoff Coefficients (C) by Surface Type
- Table C-4: Runoff Velocities (V) for Determining Time of Concentration (t_c)
- Table C-5: Manning's Roughness Coefficients for Sheet Flow (n)
- Table C-6: Depth-Duration-Interval Data (TP-40 and Hydro 35)
- Table C-7: Curve Numbers (SCS) and Percent Impervious Area

Table C-1
Equations for Calculating Rainfall Intensities

Reference Section V, Paragraph B1-a, page 2 of 8)

Storm Frequency	Intensity (i) (inches per hour)
2-Year	$65/(t_c + 8.0)^{0.806}$
5-Year	$76/(t_c + 8.5)^{0.785}$
10-Year	$80/(t_c + 8.5)^{0.763}$
25-Year	$89/(t_c + 8.5)^{0.754}$
50-Year	$98/(t_c + 8.5)^{0.745}$
100-Year	$96/(t_c + 8.0)^{0.730}$

Source: TxDOT Hydraulic Manual, 1986.

Table C-2

Runoff Coefficients (c) By Land Use Type

Reference Section V, Paragraph B1-a, page 2 of 8.

Land Use Description	Slope	Range of Values	
		From	To
Park and Open Space	Flat (0 to 2%)	0.25	0.41
	Average (2 to 7%)	0.33	0.49
	Steep (>7%)	0.73	0.53
Single Family Residential			
Lot size 5,000 to 7,000 sq. ft.	Flat (0 to 2%)	0.50	0.69
	Average (2 to 7%)	0.54	0.74
	Steep (>7%)	0.56	0.76
Lot size 7,000 to 10000 sq. ft.	Flat (0 to 2%)	0.44	0.62
	Average (2 to 7%)	0.49	0.68
	Steep (>7%)	0.52	0.71
Lot size 10,000 to 20,000 sq. ft.	Flat (0 to 2%)	0.38	0,56
	Average (2 to 7%)	0.44	0.63
	Steep (>7%)	0.47	0.66
Estate Lots (> 20,000 sq. ft.)	Flat (0 to 2%)	0.32	0.48
	Average (2 to 7%)	0.38	0.56
	Steep (>7%)	0.42	0.60
Multiple Family Residential			
Low Density (3 stories or less)	All	0.65	0.74
Medium Density (6 stories or less)	All	0.68	0.76
High Density (more than 6 stories)	All	0.71	0.80
Commercial			
Limited & General Office Sites	All	0.75	0.84
Shopping Center Sites	All	0.79	0.88
Neighborhood Business Districts	All	0.79	0.88
Office Parks	All	0.80	0.88
Central Business District	All	0.87	0.96
Industrial			
Limited (service station, restaurant)	All	0.79	0.88
General (auto sales, rental storage)	All	0.79	0.88
Heavy (parking lots, warehousing)	All	0.87	0.96

Source: City of Temple Drainage Criteria Manual

Table C-3

Runoff Coefficients (c) By Surface Type

Reference Section V, Paragraph B1-a, page 2 of 8

Surface Description	Slope	Range of Values	
		From	To
Undeveloped			
Cultivated Land	Flat (0 to 2%)	0.31	0.47
	Average (2 to 7%)	0.35	0.51
	Steep (>7%)	0.39	0.54
Pasture / Unimproved	Flat (0 to 2%)	0.25	0.41
	Average (2 to 7%)	0.33	0.49
	Steep (>7%)	0.37	0.53
Wooded	Flat (0 to 2%)	0.22	0.39
	Average (2 to 7%)	0.31	0.47
	Steep (>7%)	0.35	0.52
Floodplains	Flat (0 to 2%)	0.40	0.60
Developed Areas			
Roof Areas	All	0.92	0.97
Asphaltic Areas	All	0.90	0.95
Concrete	All	0.92	0.97
Compacted Crushed Limestone Base	All	0.80	0.90
Grass Areas (lawns, parks, etc.)			
Poor Condition	Flat (0 to 2%)	0.32	0.44
(< 50% vegetative cover)	Average (2 to 7%)	0.37	0.49
	Steep (>7%)	0.40	0.52
Fair Condition	Flat (0 to 2%)	0.25	0.37
(50 to 75% vegetative cover)	Average (2 to 7%)	0.33	0.45
	Steep (>7%)	0.37	0.49
Good Condition	Flat (0 to 2%)	0.21	0.32
(>75% vegetative cover)	Average (2 to 7%)	0.29	0.42
	Steep (>7%)	0.34	0.47

Sources: Rossmiller, R.L. "The Rational Formula Revisited"; City of Austin Drainage Criteria Manual; City of Temple Drainage Criteria Manual, as revised by the Bryan-College Station Drainage Design Guidelines Forum, March, 2005.

Table C-4

Runoff Velocities (v) for Determining Time of Concentration (t_c)[1]

Reference Section V, Paragraph B1-a, page 3 of 8.

Reach Description	Slope of Reach			
	0 to 3 %	4 to 7%	8 to 11%	>12%
	v *	v *	v*	v*
Overland or Sheet Flow				
Natural Woodlands	0 – 1.5	1.5 – 2.5	2.5 – 3.25	>3.25
Natural Grasslands	0 – 2.5	2.5 – 3.5	3.5 – 4.25	>4.25
Landscaped Areas	0 – 3.0	3.0 – 4.5	4.5 – 5.5	>5.5
Pavements	0 – 8.5	8.5 – 13.5	13.5 – 17.0	>17.0
Concentrated Flow				
Natural Channels	0 – 2.0	2.0 – 4.0	4.0 – 7.0	>7.0
Street or Gutter Flow	Use procedure in Section VI, Paragraphs A & B			
Storm Sewer	Use procedure in Section VI, Paragraph C			
Open Channels (designed)	Use procedure in Section VI, Paragraph D			
*Note: “v “ in feet per second				

[1] From the “Hydraulic Design Manual” of the Texas Department of Transportation, 2002

Table C-5

Manning’s Roughness Coefficients for Sheet Flow (n)

Reference Section V, Paragraph B1-a, page 4 of 8

Description of Surface	Roughness Coefficient (n)
Smooth surfaces (concrete, asphalt, gravel or bare soil)	0.011
Cultivated Soils	
Fallow (no residue)	0.050
Residue Cover (less than 20%)	0.060
Residue Cover (greater than 20%)	0.170
Grass	
Short grass prairie	0.150
Dense grass prairie	0.240
Bermuda grass	0.410
Range (natural)	0.130
Woods	
Light underbrush	0.400
Dense underbrush	0.800

Source: After U.S. Department of Agriculture (1986).

Table C-6

Depth-Duration-Interval Data (TP-40 and Hydro 35)

Reference Section V, Paragraph B2-b, page 5 of 8

Storm Duration	Rainfall Depth for Duration and Storm Recurrence Interval (inches)						
	2-year	5-year	10-year	25-year	50-year	100-year	USGS 500-year
5-min	0.53	0.60	0.66	0.75	0.82	0.89	-
15-min	1.15	1,33	1.46	1.66	1.82	1.98	3.0
30-min	1.68	2.00	2.24	2.59	2.87	3.14	3.6
60-min	2.20	2.68	3.02	3.52	3.91	4.30	5.8
2-hr	2.60	3.36	3.94	4.57	5.10	5.60	8.3
3-hr	2.86	3.70	4.41	5.14	5.65	6.30	9.0
6-hr	3.33	4.41	5.29	6.20	6.95	7.90	11.0
12-hr	3.80	5.25	6.28	7.42	8.45	9.50	12.5
24-hr	4.50	6.20	7.40	8.40	9.80	11.00	14.0

Source: Combination of Soil Conservation Service TP 40 and Hydro 35

Table C-7

Curve Numbers (SCS) and Percent Impervious Area[1]

Reference Section V, Paragraph B2-b, page 5 of 8

Soil Type	Pasture	Wooded	Row Crops
A	**49**	**36**	**67**
B	69	60	78
C	79	76	85
D	84	79	89

For more complete information see TR-55, Table 2-2a

Category	Percent Impervious
Land Uses	
Low Density Residential	38
Medium Density Residential	52
High Density Residential	65
Business/Commercial	85
Industrial	72

[1] Values shall be calculated for watersheds in all cases.

Hydraulic Computational Information

This portion of Appendix C provides tables and figures in support of the methodologies stipulated in Section VI of these Guidelines dealing with the application of hydraulic design principles. It includes the following Tables and Figures:

- Table C-8: Equations for Sizing Inlets on Grade
- Table C-9: Coefficient of Loss, K_j
- Table C-10: Manning's Roughness Coefficients, n
- Table C-11: Maximum Design Velocities, V
- Table C-12: Values of Entrance Loss Coefficients, K_e

Table C-8
Equations for Sizing Inlets On Grade

Reference Section VI, Paragraph B5-b, page 6 of 32

Ref. No.	Equation	Use
1	$L_x = K_c Q^{0.42} S^{0.3} \left(\frac{1}{nS_x} \right)^{0.6}$	Calculating length of curb inlet (without gutter depression) required for total interception of gutter flow.
2	$E = 1 - \left[1 - \frac{L_i}{L_T} \right]^{1.8}$	Calculating efficiency of curb inlet shorter than required length.
3	$E_o = \frac{Q_W}{Q} = 1 - \left[1 - \frac{W}{T} \right]^{2.67}$	Calculating E_o, the ratio of the frontal flow to total gutter flow for a straight roadway cross slope; used in equation 4.
4	$S_e = S_x + \frac{a}{W} E_o$	Calculating S_e to substitute for S_x in Equation 1 to determine length of curb inlet (with gutter depression) for total interception of gutter flow.
NA	**Where symbols are as follows:** E_o = Ratio of frontal flow to total gutter flow Q_W = Flow in width W, cfs Q = Total gutter flow, cfs W = Width of depressed gutter, feet T = Total spread of water in gutter, feet K_c = 0.6 (in English measure) L_x = length of curb inlet required, feet S = longitudinal slope, (ft/ft) n = Manning's roughness coefficient S_x = cross slope of road surface, (ft/ft) E = Efficiency of inlet or percentage of interception L_i = Curb-opening length, ft L_T = Curb-opening length required for 100% interception, ft S_e = equivalent cross slope, (ft/ft) a = gutter depression depth, ft	
Note:	The length of a recessed inlet is to be determined in the same manner as inlets having a depressed gutter section, because a depressed section is to be provided at the throat of the inlet but behind the curb line (Fig. C-1).	

Table C-9

Coefficient of Loss, K_j*

Reference Section VI, Paragraph C3-b, page 9 of 32

Design Condition	K_j *
Inlet on Main Line	0.50
Inlet on Main Line with Branch Lateral	0.25
Junction or Manhole on Main Line with 45 degree Branch Lateral	0.05
Junction or Manhole on Main Line with 90 degree Branch Lateral	0.25
Inlet or Manhole at Beginning of Line	1.25
Conduit on Curve for 90 degree	
Curve Radius = Diameter	0.05
Curve Radius = (2 to 8)	0.04
Curve Radius = (7 to 8)	0.25
** Where bends other than 90 Degree are used, then 90 Degree bend coefficient can be used with the following percentage factor applied:	60^o Bend – 85% 45^o Bend – 70% 22.5^o Bend – 40%

* From City of Austin Drainage Criteria Manual

Table C-10

Manning's Roughness Coefficients (n)[1]

Reference Section VI, Paragraph D2-b, page 12 of 32

Design Conditions	Coefficients Min.	Coefficients Max.
Natural Stream Channels		
Minor Streams With Fairly Regular Section, and:		
1. Some grass and weeds, little or no brush	0.030	0.035
2. Dense weeds, flow depth materially exceeds weed height	0.035	0.050
3. Some weeds, light brush on banks	0.035	0.050
4. Some weeds, heavy brush on banks	0.035	0.050
5. Some weeds, dense willows on banks	0.050	0.070
6. Trees in channels & branches submerged at high stage, increase all values above by:	0.010	0.020
Minor Streams With Irregular Section (pools, slight channel meander): use 1 to 5 above, and increase values by:	0.010	0.020
Flood Plain (adjacent to natural streams)		
Pasture: no brush, short grass	0.030	0.035
Pasture: no brush, tall grass	0.035	0.050
Heavy weeds, scattered brush	0.050	0.070
Wooded: Varies depending on undergrowth, height of foliage on trees, etc. The area of "n" = 0.10 and greater indicated extremely heavily wooded condition.	0.075	0.120
Lined Channels		
Metal corrugated	0.021	0.024
Neat concrete lined	0.012	0.018
Concrete	0.012	0.018
Concrete rubble	0.017	0.030
Grass Covered Small Channels, Shallow Depth		
No rank growth	0.035	0.045
Rank growth	0.040	0.050
Unlined Channels		
Earth, straight and uniform	0.017	0.025
Dredged	0.025	0.033
Winding and sluggish	0.022	0.030
Stony beds, weeds on bank	0.025	0.040
Earth bottom, rubble sides	0.028	0.035

[1] From "Hydraulic Design Manual" of Texas Depart of Transportation, 2002

Table C-11

Maximum Design Velocities (V) [1]

Reference: Section VI, Paragraph D3-a, page 13 of 32

Surface Treatment	Max. Design Velocity
Grass: seeded with erosion matt	4.5 ft./sec.
Grass: established sod	6.0 ft./sec.
Rubble: placed rock or concrete	10.0 ft./sec.
Impermeable: (concrete, Gunite, etc.)	15.0 ft./sec.
Gutter Flow (Sec.6, A.2.a)	10.0 ft./sec.
Channel (25-year)	Min. 2.5 ft./sec. - Max (below)
Conduit (10-year)	Min. 2.5 ft./sec. – Max. 15.0 ft./sec.
*Note: Velocities in excess of 12 feet per second shall require additional methods such as baffles, stilling basins, and/or drop structures to reduce velocities to levels stipulated.	

[1]From "Erosion and Sediment Control Guidelines for Developing Areas in Texas" by the US Soil Conservation Service.

Table C-12

Values of Entrance Loss Coefficients, K_e [1]

Reference Section VI, Paragraph F5-d, page 28 of 32

Type of Structure and Entrance Design	Value of K_e
Box, Reinforced Concrete (Submerged Entrance)	
Parallel Wing walls	0.5
Flared Wing walls	0.4
Box, Reinforced Concrete (Free Surface Flow)	
Parallel Wing walls	0.5
Flared Wing walls	0.15
Pipe, Concrete	
Projecting from fill, socket end	0.2
Projecting from fill, square cut end	0.5
With headwall or headwall and wing walls	
Socket end of pipe	0.2
Square cut end	0.5
End-section conforming to fill slope	0.5
Pipe or Pipe-Arch, Corrugated Metal	
Projecting from fill (no headwall)	0.9
Headwall or headwall & wing walls (square edge)	0.5
End-section conforming to fill slope	0.5

[1]From City of Austin Drainage Criteria Manual

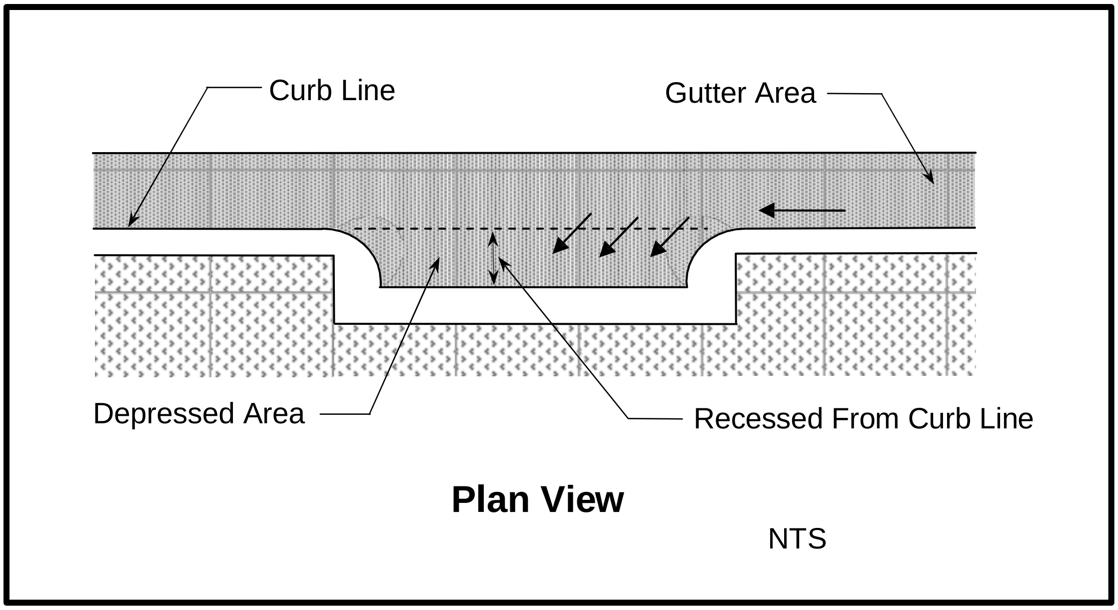


Figure C-1: Recessed Curb Inlet Diagram

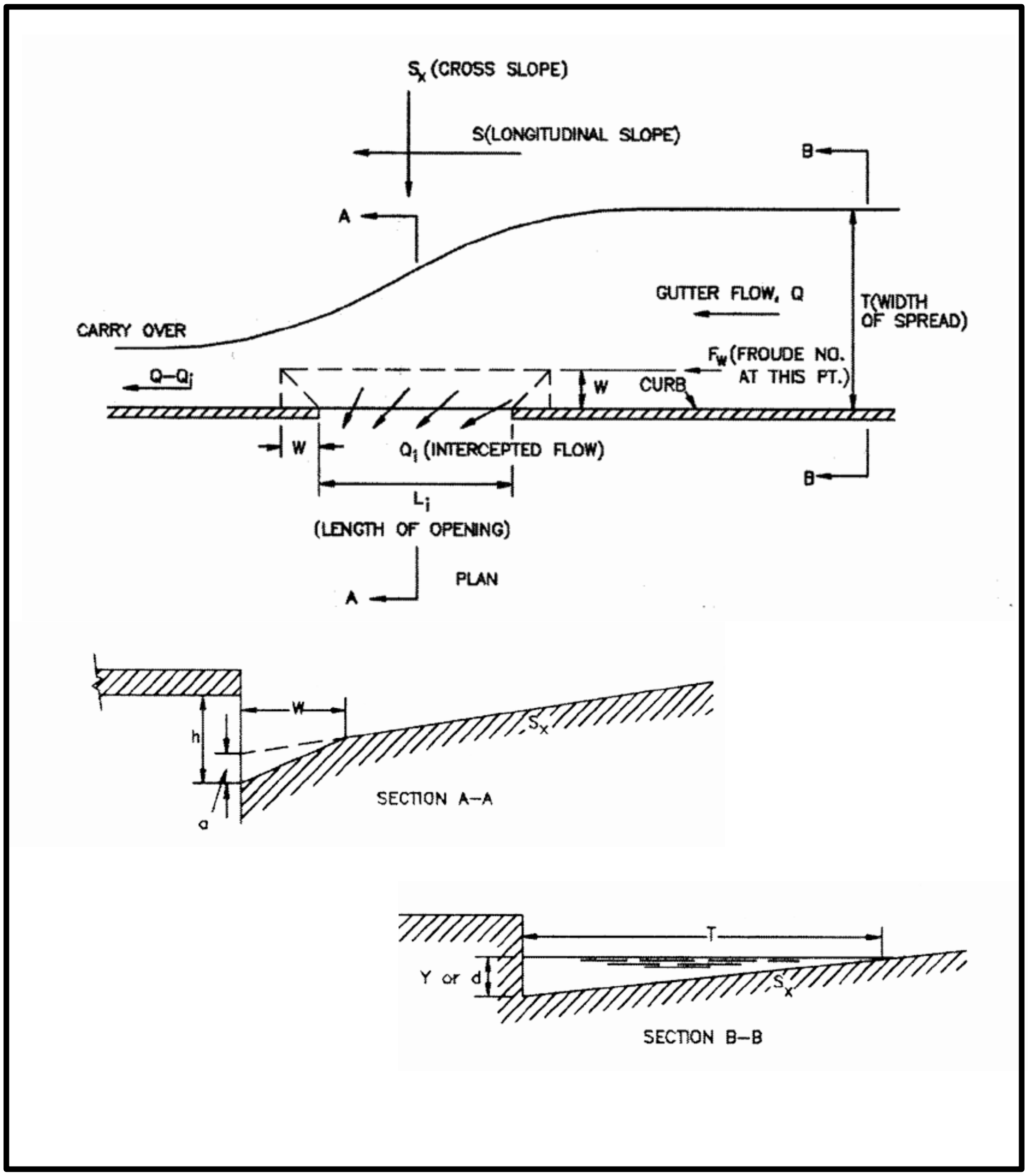


Figure C-2: Non-Recessed Curb Inlet Diagram

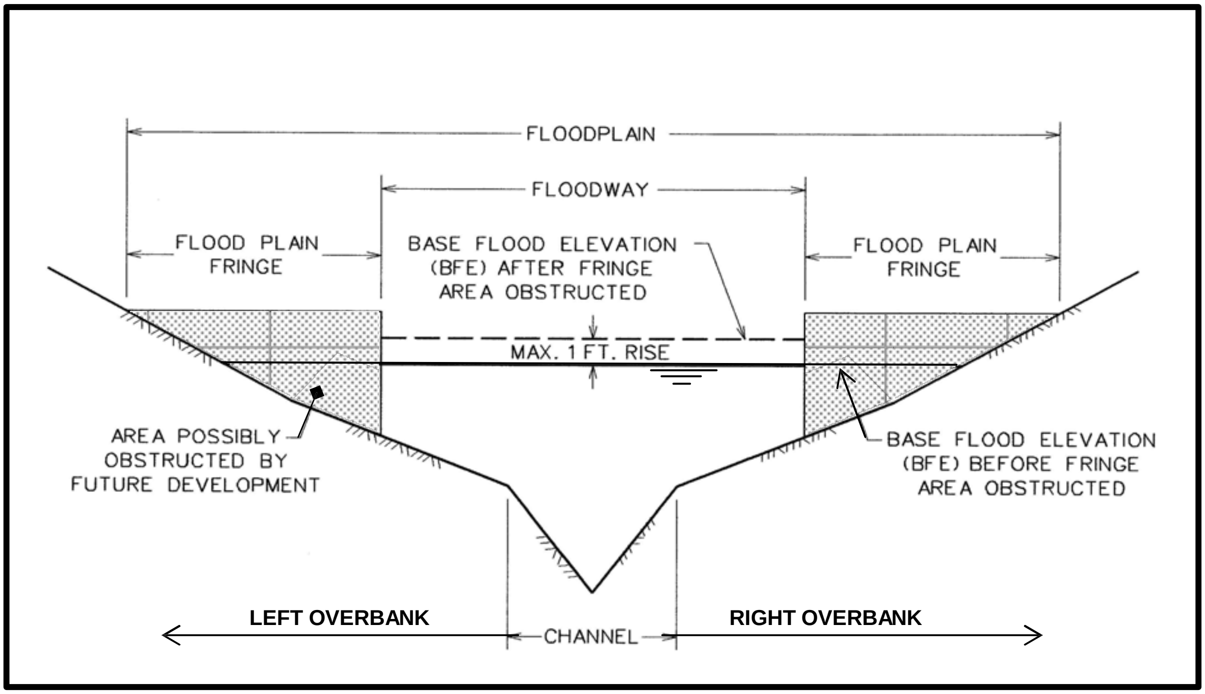


Figure C-3: Floodplain – Floodway Diagram

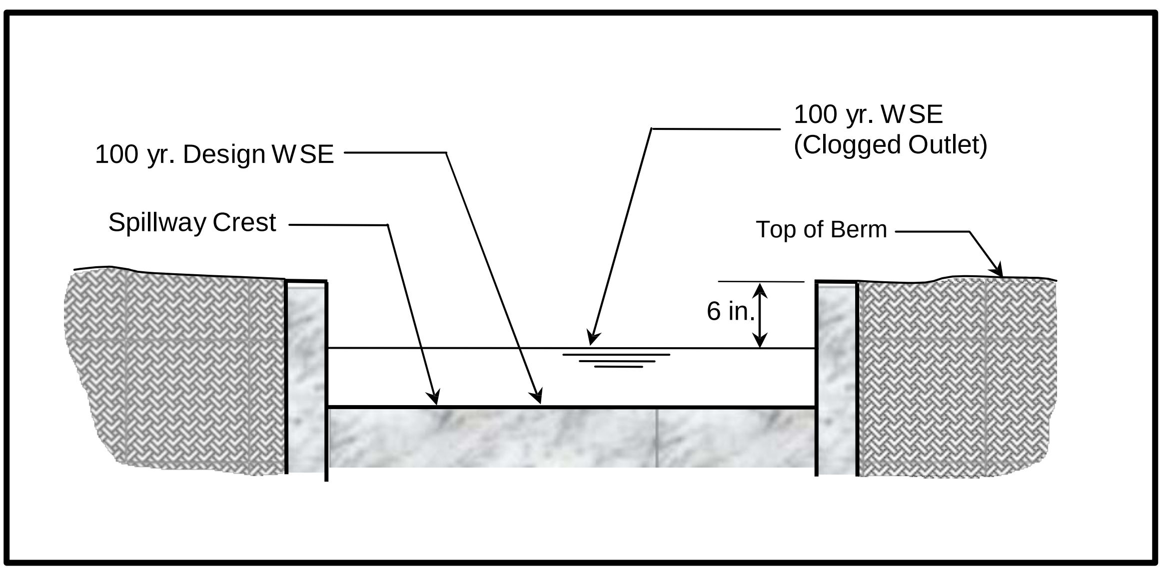
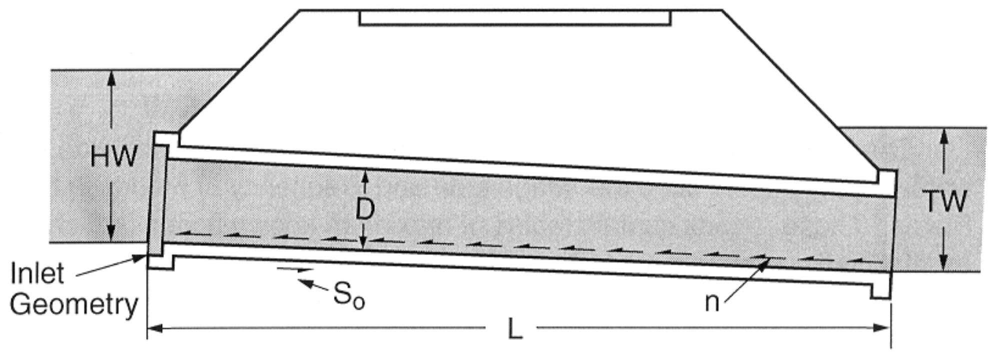


Figure C-4: Diagram of Detention Spillway Section



Where:

- **D** = inside diameter for circular pipe (ft.)
- **HW** = headwater depth at culvert entrance (ft.)
- **L** = length of culvert (ft.)
- **n** = Manning's surface roughness (dimensionless)
- **S_o** = slope of the culvert pipe (ft./ft.)
- **TW** = tailwater depth at the culvert outlet (ft.)
- **K_e** = Entrance Loss (dimensionless)

Figure C-5: Factors Influencing Culvert Discharge

Appendix D
Technical Design Summary

SECTION IX

APPENDIX D – TECHNICAL DESIGN SUMMARY

The City of Cibolo requires storm drainage design to follow these Unified Stormwater Design Guidelines. Paragraph C2 of Section III (Administration) requires submittal of a drainage report in support of the drainage plan (stormwater management plan) proposed in connection with land development projects, both site projects and subdivisions. That report may be submitted as a traditional prose report, complete with applicable maps, graphs, tables and drawings, or it may take the form of a "Technical Design Summary". The format and content for such a summary report shall be in substantial conformance with the description in this Appendix to those Guidelines. In either format the report must answer the questions (affirmative or negative) and provide, at minimum, the information prescribed in the "Technical Design Summary" in this Appendix.

The Stormwater Management Technical Design Summary Report shall include several parts as listed below. The information called for in each part must be provided as applicable. In addition to the requirements for the Executive Summary, this Appendix includes several pages detailing the requirements for a Technical Design Summary Report as forms to be completed. These are provided so that they may be copied and completed or scanned and digitized. In addition, electronic versions of the report forms may be obtained from the City. Requirements for the means (medium) of submittal are the same as for a conventional report as detailed in Section III of these Guidelines.

> **Note:** Part 1 – Executive Summary must accompany any drainage report required to be provided in connection with any land development project, regardless of the format chosen for said report.

> **Note:** Parts 2 through 6 are to be provided via the forms provided in this Appendix. Brief statements should be included in the forms as requested, but additional information should be attached as necessary.

Part 1 – Executive Summary Report

Part 2 – Project Administration

Part 3 – Project Characteristics

Part 4 – Drainage Concept and Design Parameters

Part 5 – Plans and Specifications

Part 6 – Conclusions and Attestation

STORMWATER MANAGEMENT TECHNICAL DESIGN SUMMARY REPORT

Part 1 – Executive Summary

This is to be a brief prose report that must address each of the seven areas listed below. Ideally it will include one or more paragraphs about each item.

1. Name, address, and contact information of the engineer submitting the report, and of the land owner and developer (or applicant if not the owner or developer). The date of submittal should also be included.
2. Identification of the size and general nature of the proposed project, including any proposed project phases. This paragraph should also include reference to applications that are in process with the City: plat(s), site plans, zoning requests,

or clearing/grading permits, as well as reference to any application numbers or codes assigned by the City to such request.

3. The location of the project should be described. This should identify the Named Regulatory Watershed(s) in which it is located, how the entire project area is situated therein, whether the property straddles a watershed or basin divide, the approximate acreage in each basin, and whether its position in the Watershed dictates use of detention design. The approximate proportion of the property in the city limits and within the ETJ is to be identified, including whether the property straddles city jurisdictional lines. If any portion of the property is in floodplains as described in Flood Insurance Rate Maps published by FEMA that should be disclosed.

4. The hydrologic characteristics of the property are to be described in broad terms: existing land cover; how and where stormwater drains to and from neighboring properties; ponds or wetland areas that tend to detain or store stormwater; existing creeks, channels, and swales crossing or serving the property; all existing drainage easements (or ROW) on the property, or on neighboring properties if they service runoff to or from the property.

5. The general plan for managing stormwater in the entire project area must be outlined to include the approximate size, and extent of use, of any of the following features: storm drains coupled with streets; detention / retention facilities; buried conveyance conduit independent of streets; swales or channels; bridges or culverts; outfalls to principal watercourses or their tributaries; and treatment(s) of existing watercourses. Also, any plans for reclaiming land within floodplain areas must be outlined.

6. Coordination and permitting of stormwater matters must be addressed. This is to include any specialized coordination that has occurred or is planned with other entities (local, state, or federal). This may include agencies such as Guadalupe County government, the Texas Department of Transportation, the Texas Commission for Environmental Quality, the US Army Corps of Engineers, the US Environmental Protection Agency, et al. Mention must be made of any permits, agreements, or understandings that pertain to the project.

7. Reference is to be made to the full drainage report (or the Technical Design Summary Report) which the executive summary represents. The principal elements of the main report (and its length), including any maps, drawings or construction documents, should be itemized. An example statement might be:

   > **“**One _____-page drainage report dated _______, one set of construction drawings (_____sheets) dated ________, and a ______-page specifications document dated _________ comprise the drainage report for this project.**”**

Part 2 – Project Administration	Start (Page 2.1)

Engineering and Design Professionals Information

Engineering Firm Name and Address:	Jurisdiction City: ______ Cibolo _____ Other
	Date of Submittal:
Lead Engineer's Name and Contact Info.(phone, e-mail, fax):	Other:
Supporting Engineering / Consulting Firm(s):	Other contacts:

Developer / Owner / Applicant Information

Developer / Applicant Name and Address:	Phone and e-mail:
Property Owner(s) if not Developer / Applicant (& address):	Phone and e-mail:

Project Identification

Development Name:

Is subject property a site project, a single-phase subdivision, or part of a multi-phase subdivision?

______________________________ If multi-phase, subject property is phase _____ of ______.

Legal description of subject property (phase) or **Project Area:**
(see Section II, Paragraph B-3a)

If subject property (phase) is second or later phase of a project, describe general status of all earlier phases. For most recent earlier phase Include submittal and review dates.

General Location of **Project Area,** or subject property (phase):

In City Limits? Cibolo: ___________ acres. ETJ: _____________________ acres.	Extraterritorial Jurisdiction (acreage): Cibolo: ___________ : _________ Acreage Outside ETJ: _____________

Part 2 – Project Administration	Continued (page 2.2)

Project Identification (continued)

Roadways abutting or within **Project Area** or subject property:	Abutting tracts, platted land, or built developments:
Named Regulatory Watercourse(s) & Watershed(s):	Tributary Basin(s):

Plat Information For Project or Subject Property (or Phase)

Preliminary Plat File #: ___________ Name:	Final Plat File #: _____________ Date:_____________ Status/Volume & Page:

If two plats, second name: File #: ____________

Status: Date: ___________

Zoning Information For Project or Subject Property (or Phase)

Zoning Type: Existing or Proposed? Case Code: ____________

Case Date ___________ Status:

Zoning Type: Existing or Proposed? Case Code: ____________

Case Date ___________ Status:

Stormwater Management Planning For Project or Subject Property (or Phase)

Planning Conference(s) & Date(s):	Participants:

Preliminary Report Required? ______Submittal Date ___________ Review Date _____________

Review Comments Addressed? Yes ____No _____ In Writing? _________ When? ___________

Compliance With Preliminary Drainage Report. Briefly describe (or attach documentation explaining) any deviation(s) from provisions of Preliminary Drainage Report, if any.

Part 2 – Project Administration	Continued (page 2.3)

Coordination For Project or Subject Property (or Phase)

Note: For any Coordination of stormwater matters indicated below, attach documentation describing and substantiating any agreements, understandings, contracts, or approvals.

	Dept.	Contact:	Date:	Subject:
Coordination With Other Departments of **Jurisdiction City** (Cibolo or)				

Coordination With **Non-jurisdiction City** Needed? Yes ____ No ____	Summarize need(s) & actions taken (include contacts & dates):
Coordination with Guadalupe County Needed? Yes ____ No ____	Summarize need(s) & actions taken (include contacts & dates):
Coordination with TxDOT Needed? Yes ____ No ____	Summarize need(s) & actions taken (include contacts & dates):
Other Coordination Needed? Yes ____ No ____	Summarize need(s) & actions taken (include contacts & dates):

Permits For Project or Subject Property (or Phase)

As to stormwater management, are permits required for the proposed work from any of the entities listed below? If so, summarize status of efforts toward that objective in spaces below.

Entity	**Permitted or Approved ?**	**Status of Actions** (include dates)
US Army Corps of Engineers No _____ Yes _____		
US Environmental Protection Agency No ____ Yes _____		
Texas Commission on Environmental Quality No _____ Yes _____		
Any Other Authority No _____ Yes _____		

Part 3 – Property Characteristics	Start (Page 3.1)

Nature and Scope of Proposed Work

Existing: Land proposed for development currently used, including extent of impervious cover?

Site Development Project (select all applicable)	____ Redevelopment of one platted lot, or two or more adjoining platted lots. ____ Building on a single platted lot of undeveloped land. ____ Building on two or more platted adjoining lots of undeveloped land. ____ Building on a single lot, or adjoining lots, where proposed plat will not form a new street (but may include ROW dedication to existing streets). ____ Other (explain):
Subdivision Development Project	____ Construction of streets and utilities to serve one or more platted lots. ____ Construction of streets and utilities to serve one or more proposed lots on lands represented by pending plats.
Describe Nature and Size of Proposed Project	Site projects: building use(s), approximate floor space, impervious cover ratio. Subdivisions: number of lots by general type of use, linear feet of streets and drainage easements or ROW.
Is any work planned on land that is not platted or on land for which platting is not pending? _____ No _____Yes	If yes, explain:

FEMA Floodplains

Is any part of subject property abutting a Named Regulatory Watercourse (Section II, Paragraph B1) or a tributary thereof?	No _____ Yes ______
Is any part of subject property in floodplain area of a FEMA-regulated watercourse?	No _____ Yes ____ Rate Map ___________
Encroachment(s) into Floodplain areas planned? No _____ Yes _____	Encroachment purpose(s): _____Building site(s) ______ Road crossing(s) _____ Utility crossing(s) ____ Other (explain):

If floodplain areas not shown on Rate Maps, has work been done toward amending the FEMA-Approved Flood Study to define allowable encroachments in proposed areas? Explain.

Part 3 – Property Characteristics		Continued (Page 3.2)
Hydrologic Attributes of Subject Property (or Phase)		
Has an earlier hydrologic analysis been done for larger area including subject property?		
Yes ____	Reference the study (& date) here, and attach copy if not already in City files.	
	Is the stormwater management plan for the property in substantial conformance with the earlier study? Yes _____ No ______ If not, explain how it differs.	
No ____	If subject property **is not** part of multi-phase project, describe stormwater management plan for the property in Part 4.	
	If property **is** part of multi-phase project, provide overview of stormwater management plan for **Project Area** here. In Part 4 describe how plan for subject property will comply therewith.	
Do existing topographic features on subject property store or detain runoff? ____ No ______Yes Describe them (include approximate size, volume, outfall, model, etc).		
Any known drainage or flooding problems in areas near subject property? _____No _____ Yes Identify:		
Based on location of study property in a watershed, is **Type 1 Detention** (flood control) needed? (see Table B-1 in Appendix B) ____ Detention is required. _____ Need must be evaluated. _____ Detention not required.		
If the need for **Type 1 Detention** must be evaluated:	What decision has been reached? By whom?	
	How was determination made?	

Part 3 – Property Characteristics		Continued (Page 3.3)
Hydrologic Attributes of Subject Property (or Phase) (continued)		
Does subject property straddle a Watershed or Basin divide? _____ No _____Yes If yes, describe splits below. In Part 4 describe design concept for handling this.		
Watershed or Basin	**Larger acreage**	**Lesser acreage**
Above-Project Areas(Section II, Paragraph B3-a)		
Does Project Area (project or phase) receive runoff from upland areas? ____ No _____ Yes Size(s) of area(s) in acres: 1) ________ 2) __________ 3) ________ 4) _________		
Flow Characteristics (each instance) (overland sheet, shallow concentrated, recognizable concentrated section(s), small creek (non-regulatory), regulatory Watercourse or tributary);		
Flow determination: Outline hydrologic methods and assumptions:		
Does storm runoff drain from public easements or ROW onto or across subject property? _____ No _____ Yes If yes, describe facilities in easement or ROW:		
Are changes in runoff characteristics subject to change in future? Explain		
Conveyance Pathways (Section II, Paragraph C2)		
Must runoff from study property drain across lower properties before reaching a Regulatory Watercourse or tributary? _____ No _____ Yes		
Describe length and characteristics of each conveyance pathway(s). Include ownership of property(ies).		

Part 3 – Property Characteristics		Continued (Page 3.4)
Hydrologic Attributes of Subject Property (or Phase) (continued)		
Conveyance Pathways (continued)		
	Do drainage easements exist for any part of pathway(s)? ____No ____Yes	If yes, for what part of length? ______% Created by? ____ plat, or _____instrument. If instrument(s), describe their provisions.
Pathway Areas	Where runoff must cross lower properties, describe characteristics of abutting lower property(ies). (Existing watercourses? Easement or Consent acquired?)	
Nearby Drainage Facilities	Describe any built or improved drainage facilities existing near the property (culverts, bridges, lined channels, buried conduit, swales, detention ponds, etc).	
	Do any of these have hydrologic or hydraulic influence on proposed stormwater design? _____ No _____ Yes If yes, explain:	

Part 4 – Drainage Concept and Design Parameters	Start (Page 4.1)
Stormwater Management Concept	
Discharge(s) From Upland Area(s)	
If runoff is to be received from upland areas, what design drainage features will be used to accommodate it and insure it is not blocked by future development? Describe for each area, flow section, or discharge point.	
Discharge(s) To Lower Property(ies) (Section II, Paragraph E1)	
Does project include drainage features (existing or future) proposed to become public via platting? _____No ____ Yes Separate Instrument? _______No ______Yes	
Per Guidelines reference above, how will runoff be discharged to neighboring property(ies)?	____ Establishing Easements (Scenario 1) ____ Pre-development Release (Scenario 2) ____ Combination of the two Scenarios
Scenario 1: If easements are proposed, describe where needed, and provide status of actions on each. (Attached Exhibit #______)	
Scenario 2: Provide general description of how release(s) will be managed to pre-development conditions (detention, sheet flow, partially concentrated, etc.). (Attached Exhibit #______)	
Combination: If combination is proposed, explain how discharge will differ from pre-development conditions at the property line for each area (or point) of release.	
If Scenario 2, or Combination are to be used, has proposed design been coordinated with owner(s) of receiving property(ies)? _____ No _____ Yes Explain and provide documentation.	

Part 4 – Drainage Concept and Design Parameters	Continued (Page 4.2)
Stormwater Management Concept (continued)	
Within Project Area Of Multi-Phase Project	
Will project result in shifting runoff between Basins or between Watersheds? _____ No _____Yes	Identify gaining Basins or Watersheds and acres shifting:
	What design and mitigation is used to compensate for increased runoff from gaining basin or watershed?
How will runoff from **Project Area** be mitigated to pre-development conditions? Select any or all of 1, 2, and/or 3, and explain below.	1. _____With facility(ies) involving other development projects. 2. _____Establishing features to serve overall Project Area. 3. _____On phase (or site) project basis within Project Area.
1. Shared facility (type & location of facility; design drainage area served; relationship to size of Project Area): (Attached Exhibit #______)	
2. For Overall Project Area (type & location of facilities): (Attached Exhibit #______)	
3. By phase (or site) project: Describe planned mitigation measures for phases (or sites) in subsequent questions of this Part.	
Are Special Designs Planned? _____ No _____ Yes	Are aquatic ecosystems proposed? _____ No _____ Yes In which phase(s) or project(s)?
	Are other Best Management Practices for reducing stormwater pollutants proposed? ____ No ____ Yes Summarize type of BMP and extent of use:
	If design of any runoff-handling facilities deviate from provisions of B-CS Technical Specifications, check type facility(ies) and explain in later questions. ____ Detention elements _____ Conduit elements ____ Channel features ____ Swales _____ Ditches ______ Inlets _____ Valley gutters _____ Outfalls ____ Culvert features ______Bridges _________________Other

Part 4 – Drainage Concept and Design Parameters	Continued (Page 4.3)
Stormwater Management Concept (continued)	
Within Project Area Of Multi-Phase Project (continued)	
Will Project Area include bridge(s) or culvert(s)? ____ No _____ Yes Identify type and general size and In which phase(s).	
If detention/retention serves (will serve) overall Project Area, describe how it relates to subject phase or site project (physical location, conveyance pathway(s), construction sequence):	
Within Or Serving Subject Property (Phase, or Site)	
If property part of larger Project Area, is design in substantial conformance with earlier analysis and report for larger area? _____ Yes ______ No, then summarize the difference(s):	
Identify whether each of the types of drainage features listed below are included, extent of use, and general characteristics.	

Are roadside ditches used? ____ No ____ Yes	Typical shape?	Surfaces?	
	Steepest side slopes:	Usual front slopes:	Usual back slopes:
	Flow line slopes: least__________ typical_________ greatest__________	Typical distance from travelway: (Attached Exhibit #_____)	
	Are longitudinal culvert ends in compliance with Cibolo Specifications? ______Yes _______No, then explain:		
Are streets with curb and gutter used? ____ No ____ Yes	At intersections or otherwise, do valley gutters cross arterial or collector streets? ______ No _____Yes If yes explain:		
	Are valley gutters proposed to cross any street away from an intersection? ____No ____Yes Explain: (number of locations?)		

Part 4 – Drainage Concept and Design Parameters			Continued (Page 4.4)
Stormwater Management Concept (continued)			
Within Or Serving Subject Property (Phase, or Site) (continued)			
	Are streets with curb and gutter used? (continued)	Gutter line slopes: Least ___________ Usual _________ Greatest ____________	
		Are inlets recessed on arterial and collector streets? _____Yes ______ No If “no”, identify where and why.	
		Will inlets capture 10-year design stormflow to prevent flooding of intersections (arterial with arterial or collector)? ____ Yes _____ No If no, explain where and why not.	
		Will inlet size and placement prevent exceeding allowable water spread for 10-year design storm throughout site (or phase)? _____Yes _____ No If no, explain.	
		Sag curves: Are inlets placed at low points? ____ Yes ______No Are inlets and conduit sized to prevent 100-year storm-flow from ponding at greater than 24 inches? ____ Yes _____ No Explain “no” answers.	
		Will 100-yr stormflow be contained in combination of ROW and buried conduit on whole length of all streets? _____Yes _____No If no, describe where and why.	
		Do designs for curb, gutter, and inlets comply with Cibolo Specifications? ______Yes _____No If not, describe difference(s) and attach justification.	
	Is storm drain system used? ___ No ___ Yes	Are any 12-inch laterals used? _____ No _____Yes Identify length(s) and where used.	
		Pipe runs between system access points (feet):	Typical ___________ Longest ___________
		Are junction boxes used at each bend? ____ Yes ______No If not, explain where and why.	
		Are downstream soffits at or below upstream soffits? Yes ____ No _____If not, explain where and why:	Least amount that hydraulic grade line is below gutter line (system-wide):

<table>
<tr><td colspan="4"><u>Part 4 – Drainage Concept and Design Parameters</u></td><td>Continued (Page 4.5)</td></tr>
<tr><td colspan="5">Stormwater Management Concept (continued)</td></tr>
<tr><td colspan="5">Within Or Serving Subject Property (Phase, or Site) (continued)</td></tr>
<tr><td rowspan="10"></td><td rowspan="5">Storm drain system (continued)
(on separate sheet provide same info. for more instances)</td><td rowspan="5">Outfall(s)</td><td colspan="2">Describe watercourse(s), or system(s) receiving system discharge(s) below (include design discharge velocity, and angle between converging flow lines).</td></tr>
<tr><td colspan="2">1) Watercourse (or system), velocity, and angle?</td></tr>
<tr><td colspan="2">2) Watercourse (or system), velocity, and angle?</td></tr>
<tr><td colspan="2">3) Watercourse (or system), velocity, and angle?</td></tr>
<tr><td colspan="2">For each outfall above, what measures are taken to prevent erosion or scour of receiving and all facilities at juncture?
1)
2)
3)</td></tr>
<tr><td rowspan="5">Are swales used to drain streets?
____ No ____ Yes</td><td colspan="3">Are swale(s) situated along property lines between properties? ____ No ____ Yes
Number of instances: ______ For each instance answer the following questions.</td></tr>
<tr><td colspan="3">Surface treatments (including low-flow flumes if any):</td></tr>
<tr><td colspan="3">Flow line slopes (minimum and maximum):</td></tr>
<tr><td colspan="3">Outfall characteristics for each (velocity, convergent angle, & end treatment).</td></tr>
<tr><td colspan="3">Will 100-year design storm runoff be contained within easement(s) or platted drainage ROW in all instances? _____Yes _____No If “no” explain:</td></tr>
</table>

Part 4 – Drainage Concept and Design Parameters Continued (Page 4.6)

Stormwater Management Concept (continued)

Within Or Serving Subject Property (Phase, or Site) (continued)

Roadside Ditches

Are roadside ditches used? _____No _____Yes If so, provide the following:
Is 25-year flow contained with 6 inches of freeboard throughout? _____Yes _____ No
Are top of banks separated from road shoulders 2 feet or more? ____Yes ____No
Are all ditch sections trapezoidal and at least 1.5 feet deep? ____Yes ____ No

For any "no" answers provide location(s) and explain:

Are swale/conduit combinations used in lieu of open channels? ___ No ___ Yes (on separate sheet provide same information for any additional instances)

If conduit is beneath a swale, provide the following information (each instance).

Instance 1 Describe general location, approximate length:

Is 100-year design flow contained in conduit/swale combination? ____ Yes ____ No
If "no" explain:

Space for 100-year storm flow? ROW _____ Easement _____ Width __________

Swale Surface type, minimum and maximum slopes:

Conduit Type and size, minimum and maximum slopes, design storm:

Inlets Describe how conduit is loaded (from streets/storm drains, inlets by type):

Access Describe how maintenance access is provided (to swale, into conduit):

Instance 2 Describe general location, approximate length:

Is 100-year design flow contained in conduit/swale combination? ____ Yes ____ No
If "no" explain:

Space for 100-year storm flow? ROW _____ Easement _____ Width __________

Swale Surface type, minimum and maximum slopes:

Conduit Type and size, minimum and maximum slopes, design storm:

Inlets Describe how conduit is loaded (from streets/storm drains, inlets by type):

Access Describe how maintenance access is provided (to swale, into conduit):

<table>
<tr><td colspan="3"><u>Part 4 – Drainage Concept and Design Parameters</u></td><td>Continued (Page 4.7)</td></tr>
<tr><td colspan="4">Stormwater Management Concept (continued)</td></tr>
<tr><td colspan="4">Within Or Serving Subject Property (Phase, or Site) (continued)</td></tr>
<tr><td rowspan="8"></td><td rowspan="8">Will swales without buried conduit receive runoff from public ROW or easements? ____ No ____ Yes. Explain</td><td colspan="2">If “yes” provide the following information for each instance:</td></tr>
<tr><td colspan="2"><u>Instance 1</u> Describe general location, approximate length, surfacing:</td></tr>
<tr><td colspan="2">Is 100-year design flow contained in swale? ____ Yes ____ No Is swale wholly within drainage ROW? _____Yes ______ No Explain “no” answers:</td></tr>
<tr><td colspan="2"><u>Access</u> Describe how maintenance access is provide:</td></tr>
<tr><td colspan="2"><u>Instance 2</u> Describe general location, approximate length, surfacing:</td></tr>
<tr><td colspan="2">Is 100-year design flow contained in swale? ____ Yes ____ No Is swale wholly within drainage ROW? _____Yes _____ No Explain “no” answers:</td></tr>
<tr><td colspan="2"><u>Access</u> Describe how maintenance access is provided:</td></tr>
<tr><td colspan="2"><u>Instance 3, 4, etc.</u> If swales are used in more than two instances, attach sheet providing all above information for each instance.</td></tr>
<tr><td rowspan="2"></td><td rowspan="2">Channel improvements proposed? ____ No ____ Yes Explain</td><td colspan="2"><u>“New” channels:</u> Will any area(s) of concentrated flow be channelized (deepened, widened, or straightened) or otherwise altered? ___ No _____Yes If only slightly shaped, see “Swales” in this Part. If creating side banks, provide information below.</td></tr>
<tr><td colspan="2">Will design replicate natural channel? ____Yes ____ No If “no”, for each instance describe section shape & area, flow line slope (min. & max.), surfaces, and 100-year design flow, and amount of freeboard:
Instance 1:

Instance 2:

Instance 3:</td></tr>
</table>

Part 4 – Drainage Concept and Design Parameters		Continued (Page 4.8)
Stormwater Management Concept (continued)		
Within Or Serving Subject Property (Phase, or Site) (continued)		
	Channel Improvements (continued)	**Existing channels (small creeks):** Are these used? _____ No ____ Yes If "yes" provide the information below.
		Will small creeks and their floodplains remain undisturbed? ____ Yes ____No How many disturbance instances? ______ Identify each planned location:
		For each location, describe length and general type of proposed improvement (including floodplain changes):
		For each location, describe section shape & area, flow line slope (min. & max.), surfaces, and 100-year design flow.
		Watercourses (and tributaries): Aside from fringe changes, are Regulatory Watercourses proposed to be altered? _____ No _____Yes Explain below.
		Submit full report describing proposed changes to Regulatory Watercourses. Address existing and proposed section size and shape, surfaces, alignment, flow line changes, length affected, and capacity, and provide full documentation of analysis procedures and data. Is full report submitted? _____ Yes _____ No If "no" explain:
		All Proposed Channel Work: For all proposed channel work, provide information requested in next three boxes.
		If design is to replicate natural channel, identify location and length here, and describe design in Special Design section of this Part of Report.
		Will 100-year flow be contained with one foot of freeboard? ____ Yes ____No If not, identify location and explain:
		Are ROW / easements sized to contain channel and required maintenance space? ____Yes ____No If not, identify location(s) and explain:

Part 4 – Drainage Concept and Design Parameters						Continued (Page 4.9)
Stormwater Management Concept (continued)						
Within Or Serving Subject Property (Phase, or Site) (continued)						
	Are Detention Facilities Proposed? ____ No ____ Yes	How many facilities for subject property project? ____ For each provide info. below.				
		For each dry-type facility:	Facility 1		Facility 2	
		Acres served & design volume + 10%				
		100-yr volume: free flow & plugged				
		Design discharge (10 yr & 25 yr)				
		Spillway crest at 100-yr WSE?	____ yes	_____no	____ yes	_____no
		Berms 6 inches above plugged WSE?	____ yes	_____no	____ yes	_____no
		Explain any "no" answers:				
		For each facility what is 25-yr design Q, and design of outlet structure? Facility 1: Facility 2:				
		Do outlets and spillways discharge into a public facility in easement or ROW? Facility 1: ____ Yes ____No Facility 2: ____Yes _____ No If "no" explain:				
		For each, what is velocity of 25-yr design discharge at outlet? & at spillway? Facility 1: ___________& ___________ Facility 2: ___________& ____________ Are energy dissipation measures used? _____No ____Yes Describe type and location:				
		For each, is spillway surface treatment other than concrete? Yes or no, and describe: Facility 1: Facility 2:				
		For each, what measures are taken to prevent erosion or scour at receiving facility? Facility 1: Facility 2:				
		If berms are used give heights, slopes and surface treatments of sides. Facility 1: Facility 2:				

Part 4 – Drainage Concept and Design Parameters		Continued (Page 4.10)
Stormwater Management Concept (continued)		
Within Or Serving Subject Property (Phase, or Site) (continued)		
Detention Facilities (continued)	Do structures comply with Cibolo Specifications? Yes or no, and explain if "no": Facility 1; Facility 2:	
	For additional facilities provide all same information on a separate sheet.	
	Are parking areas to be used for detention? ____ No _____ Yes What is maximum depth due to required design storm?	
Are culverts used at private crossings? ___ No ___ Yes	**Roadside Ditches**: Will culverts serve access driveways at roadside ditches? ____No _____Yes If "yes", provide information in next two boxes.	
	Will 25-yr. flow pass without flowing over driveway in all cases? _____ Yes ____ No Without causing flowing or standing water on public roadway? _____ Yes _____ No Designs & materials comply with Cibolo Technical Specifications? _____ Yes _____ No Explain any "no" answers:	
	Are culverts parallel to public roadway alignment? ____ Yes ____ No Explain:	
	Creeks at Private Drives: Do private driveways, drives, or streets cross drainage ways that serve Above-Project areas or are in public easements/ ROW? _____ No ____ Yes If "yes" provide information below.	
	How many instances? _______ Describe location and provide information below. Location 1: Location 2: Location 3:	

For each location enter value for:	1	2	3
Water design on travelway at 10-year flow?			
Water depth on ROW/travelway at 25-year flow?			
Water depth on ROW/travelway at 100-year flow?			

For more instances describe location and same information on separate sheet.

Part 4 – Drainage Concept and Design Parameters | Continued (Page 4.11)

Stormwater Management Concept (continued)

Within Or Serving Subject Property (Phase, or Site) (continued)

Are culverts used at public roadway crossings? _____ No _____ Yes
(for more instances of any type describe location and same information on separate sheet)

Named Regulatory Watercourses (& Tributaries): Are culverts proposed on these facilities? _____ No _____Yes, then provide full report documenting assumptions, criteria, analysis, computer programs, and study findings that support proposed design(s). Is report provided? ____ Yes _____No If "no", explain:

Arterial or Major Collector Streets: Will culverts serve these types of roadways? _____ No _____Yes How many instances? ________ For each identify the location and provide the information below.

Instance 1:

Instance 2:

Instance 3:

Yes or No for the 100-year design flow:	1	2	3
Headwater WSE 1 foot below lowest curb top?			
Spread of headwater within ROW or easement?			
Is velocity limited per conditions (Table C-11)?			

Explain any "no" answer(s):

Minor Collector or Local Streets: Will culverts serve these types of streets? ____ No _____ Yes How many instances? _______ for each identify the location and provide the information below:

Instance 1:

Instance 2:

Instance 3:

For each instance enter value, or "yes" / "no" for:	1	2	3
Design yr. headwater WSE 1 ft. below curb top?			
100-yr. max. depth at street crown 2 feet or less?			
Product of velocity (fps) & depth at crown (ft) = ?			
Is velocity limited per conditions (Table C-11)?			
Limit of downstream analysis (feet)?			

Explain any "no" answers:

Part 4 – Drainage Concept and Design Parameters		Continued (Page 4.12)
Stormwater Management Concept (continued)		
Within Or Serving Subject Property (Phase, or Site) (continued)		
	Culverts (continued)	**All Proposed Culverts:** For all proposed culvert facilities (except driveway/roadside ditch intersects) provide information requested in next eight boxes.
		Do culverts and travelways intersect at 90 degrees? ____Yes ____ No If not, identify location(s) and intersect angle(s), and justify the design(s):
		Does drainage way alignment change within or near limits of culvert and surfaced approaches thereto? ____No _____Yes If "yes" identify location(s), describe change(s), and justification:
		Are flumes or conduit to discharge into culvert barrel(s)? _____No ____ Yes If yes, identify location(s) and provide justification:
		Are flumes or conduit to discharge into or near surfaced approaches to culvert ends? ____ No _____Yes If "yes" identify location(s), describe outfall design treatment(s):
		Is scour/erosion protection provided to ensure long term stability of culvert structural components, and surfacing at culvert ends? ____ Yes _____No If "no" Identify locations and provide justification(s):
		Will 100-yr flow and spread of backwater be fully contained in street ROW, and/or drainage easements/ ROW? ____Yes ____ No if not, why not?
		Do appreciable hydraulic effects of any culvert extend downstream or upstream to neighboring land(s) not encompassed in subject property? _____ No _____Yes If "yes" describe location(s) and mitigation measures:
		Are all culvert designs and materials in compliance with Cibolo Tech. Specifications? _____ Yes ____No If not, explain in Special Design Section of this Part.

Part 4 – Drainage Concept and Design Parameters			Continued (Page 4.13)
Stormwater Management Concept (continued)			
Within Or Serving Subject Property (Phase, or Site) (continued)			
	Bridge(s)	Is a bridge included in plans for subject property project? ____ No ___ Yes If “yes” provide the following information.	
		Name(s) and functional classification of the roadway(s)?	
		What drainage way(s) is (are) to be crossed?	
		A full report supporting all aspects of the proposed bridge(s) (structural, geotechnical, hydrologic, and hydraulic factors) must accompany this summary report. Is the report provided? ____Yes ____ No If “no” explain:	
	Water Quality	Is a Stormwater Pollution Prevention Plan (SWPPP) established for project construction? ____No _____Yes	Provide a general description of planned techniques:
	Special Designs – Non-Traditional Methods		
	Are any non-traditional methods (aquatic ecosystems, wetland-type detention, natural stream replication, BMPs for water quality, etc.) proposed for any aspect of subject property project? ____No _____Yes If “yes” list general type and location below.		
	Provide full report about the proposed special design(s) including rationale for use and expected benefits. Report must substantiate that stormwater management objectives will not be compromised, and that maintenance cost will not exceed those of traditional design solution(s). Is report provided? ____ Yes _____ No If “no” explain:		

Part 4 – Drainage Concept and Design Parameters	Continued (Page 4.14)

Stormwater Management Concept (continued)

Within Or Serving Subject Property (Phase, or Site) (continued)

Special Designs – Deviation From Cibolo Technical Specifications

If any design(s) or material(s) of traditional runoff-handling facilities deviate from provisions of B-CS Technical Specifications, check type facility(ies) and explain by specific detail element.

____ Detention elements ______Drain system elements ______ Channel features
____ Culvert features ______Swales ______ Ditches ____ Inlets ______Outfalls
____ Valley gutters ____ Bridges (explain in bridge report)

In table below briefly identify specific element, justification for deviation(s).

Specific Detail Element	Justification for Deviation (attach additional sheets if needed)
1)	
2)	
3)	
4)	
5)	

Have elements been coordinated with the City Engineer or her/his designee? For each item above provide “yes” or “no”, action date, and staff name:

1)
2)
3)
4)
5)

Design Parameters

Hydrology

Is a map(s) showing all Design Drainage Areas provided? ______Yes ______No

Briefly summarize the range of applications made of the Rational Formula:

What is the size and location of largest Design Drainage Area to which the Rational Formula has been applied? ______ acres Location (or identifier):

Part 4 – Drainage Concept and Design Parameters

Continued (Page 4.15)

Design Parameters (continued)

Hydrology (continued)

In making determinations for time of concentration, was segment analysis used?
___ No ____Yes In approximately what percent of Design Drainage Areas? _______ %

As to intensity-duration-frequency and rain depth criteria for determining runoff flows, were any criteria other than those provided in these Guidelines used? ____ No _____Yes If "yes" identify type of data, source(s), and where applied:

For each of the stormwater management features listed below identify the storm return frequencies (year) analyzed (or checked), and that used as the basis for design.

Feature	Analysis Year(s)	Design Year
Storm drain system for arterial and collector streets		
Storm drain system for local streets		
Open channels		
Swale/buried conduit combination in lieu of channel		
Swales		
Roadside ditches and culverts serving them		
Detention facilities: spillway crest and its outfall		
Detention facilities: outlet and conveyance structure(s)		
Detention facilities: volume when outlet plugged		
Culverts serving private drives or streets		
Culverts serving public roadways		
Bridges: provide in bridge report.		

Hydraulics

What is the range of design flow velocities, as outlined below?

Design flow velocities;	Gutters	Conduit	Culverts	Swales	Channels
Highest (feet per second)					
Lowest (feet per second)					

Streets and Storm Drain Systems Provide the summary information outlined below:

Roughness coefficients used: For street gutters: ________

For conduit type(s) ________________ _______________ Coefficients: ______ ________

Part 4 – Drainage Concept and Design Parameters	Continued (Page 4.16)
Design Parameters (continued)	
Hydraulics (continued)	
	Street and Storm Drain Systems (continued)
	For the following, are assumptions other than allowable per Guidelines? Inlet coefficients? ____ No ____ Yes Head and friction losses ____ No ____ Yes Explain any "yes" answer:
	In conduit is velocity generally increased in the downstream direction? ____Yes ____ No Are elevation drops provided at inlets, manholes, and junction boxes? ____ Yes ____ No Explain any "no" answers:
	Are hydraulic grade lines calculated and shown for design storm? ____ Yes ____ No For 100-year flow conditions? ____Yes ____ No Explain any "no" answers:
	What tailwater conditions were assumed at outfall point(s) of the storm drain system? Identify each location and explain:
	Open Channels If a HEC analysis is utilized, does it follow Sec VI.F.5.a? ____Yes ____ No
	Outside of straight sections, is flow regime within limits of sub-critical flow? ____ Yes ____ No If "no" list locations and explain:
	Culverts If plan sheets do not provide the following for each culvert, describe it here.
	For each design discharge, will operation be outlet (barrel) control or inlet control?
	Entrance, friction and exit losses:
	Bridges Provide all in bridge report

Part 4 – Drainage Concept and Design Parameters	Continued (Page 4.17)
Design Parameters (continued)	
Computer Software	
What computer software has been used in the analysis and assessment of stormwater management needs and/or the development of facility designs proposed for subject property project? List them below, being sure to identify the software name and version, the date of the version, any applicable patches and the publisher	

Part 5 – Plans and Specifications

Requirements for submittal of construction drawings and specifications do not differ due to use of a Technical Design Summary Report. See Section III, Paragraph C3.

Part 6 – Conclusions and Attestation

Conclusions

Add any concluding information here:

Attestation

Provide attestation to the accuracy and completeness of the foregoing 6 Parts of this Technical Design Summary Drainage Report by signing and sealing below.

"This report (plan) for the drainage design of the development named in Part B was prepared by me (or under my supervision) in accordance with provisions of the Cibolo Unified Stormwater Design Guidelines for the owners of the property. All licenses and permits required by any and all state and federal regulatory agencies for the proposed drainage improvements have been issued or fall under applicable general permits."

(Affix Seal)

____

Licensed Professional Engineer

*State of Texas PE No.*________________

Appendix E
Best Management Practices

APPENDIX E – BEST PRACTICES

As defined in Section VII of these Guidelines, improving stormwater quality is a worthy objective. At key points, the Guidelines encourage special designs aimed at improving the quality of stormwater discharged into the region's major streams and waterways. Specific details for such designs are not stipulated. Rather, applications are left to the creativity of qualified engineers and environmental specialists who serve the development community.

This Appendix is provided in order to facilitate and foster design solutions that will help improve water quality. The effectiveness of the techniques outlined herein is very dependent on proper application and implementation, and is in no way assured. Likewise their use does not assure achieving public safety objectives, and can work against those objectives if improperly conceived or deployed.

Special designs may propose using any of the examples outlined herein or other techniques that may have been implemented in other jurisdictions. It is highly recommended that any special design concepts be carefully coordinated with the City Engineer or his/her designee as early as possible in design processes. It shall be the designers' responsibility to substantiate that the special design does not compromise public safety objectives or aggravate long term maintenance requirements.

"Best Management Practices"

In their publication "National Menu of Best Management Practices For Storm Water Phase II", the US Environmental Protection Agency (EPA) has advanced a number of concepts for managing urban stormwater runoff in a manner that will enhance water quality. The techniques are intended to provide guidance to regulated small MS-4's. This Appendix provides a brief introduction to several of those techniques. They are offered only as examples. There is no requirement to use them, nor are they specifically recommended over other potential design solutions. Likewise, designers should not limit their thinking to only these examples. Specific design criteria are provided in the Erosion and Sedimentation section of the Cibolo Design & Construction Manual.

All of the techniques offered by the EPA have been used at various locations and have been scientifically evaluated for their general effectiveness. The specific chemical or physical effectiveness of the techniques is beyond the scope of these Guidelines, as are their advantages and disadvantage in terms of initial cost, comparative costs, or maintenance ramifications. Nevertheless, these later issues must be addressed in technical reports substantiating special design proposals. The designers' attention is directed to the aforementioned publication for the information necessary to implement these and other techniques.

Retention / Irrigation Basins

Retention refers to the idea of capturing stormwater and retaining it, as opposed to simply collecting it and metering its release at some pre-determined flow rate. As suggested by the title, the concept of this technique is to collect runoff into a holding pond and then draw from it to irrigate landscaped areas. The intent is to replicate natural situations where the majority of rainfall is infiltrated into the soil or underlying groundwater, and pollutants are captured by soils. In addition, particles settle while the water is pooled.

Extended Detention Basins

A traditional detention facility captures storm flow and releases it at a pre-determined rate, one associated with pre-development conditions, with no particular consideration for water quality objectives. An "extended detention basin" functions in a similar way but is designed to release the collected water at a much slower rate, one that causes the water to remain pooled much longer, usually on the order of 24 hours. This allows time for suspended solids to settle, and can derive other water quality benefits. Such a facility should serve no more than 100 acres, and generally requires a slower release rate and a larger storage volume than a traditional detention facility.

Grassy Swales

A grassy swale is a specially designed channel. With very flat side slopes (4:1 or flatter), it is wider than it is deep. The flow line slope should be between one percent and five percent, and the surfaces must be covered with vegetation, generally close-growing, water-resistant grasses. The idea is simple: as runoff flows over and through the grass at a shallow depth and slow rate, particles tend to settle and biological uptake of pollutants tends to occur.

Vegetative Filter Strips

As suggested by the name, this technique involves long strips of vegetated area placed so that runoff will traverse their length in route to lower areas. The idea is to bring runoff to the strips in broad sheet flow or in uniform shallow overland flow, not in a concentrated manner. As stormwater moves through the strip(s) in very shallow flow at a slow rate, the vegetation tends to cause particles to settle and biological filtration of pollutants.

Sand Filter Systems

These systems can vary widely in their design but in any case require carefully specified and constructed components in order to be effective. Generally, two chambers are required, one for sedimentation and another for filtration. Runoff first enters the sedimentation chamber where larger solids are collected. Next it seeps through the sand bed in the filtration chamber. There, a specially designed sand bed composed of sand, gravel, and filter fabric in just the right combinations and having just the right physical characteristics, captures a range of other pollutants. Water is finally released through perforated collection pipe(s) situated beneath the sand bed system.

A "full sedimentation" system includes a wall with a riser pipe between the two chambers. This type requires the first chamber to be sized for the entire design capture volume. A "partial sedimentation" system includes a porous separation between the two chambers so larger solids may not pass into the filtration chamber. In this type, the two chambers together are sized for the entire design capture volume.

Wet Basins

In simplest terms a wet basin is designed to retain a pool of water year-round. Whereas a traditional detention facility has an outlet near its bottom, a wet basin has an outlet located near its top. With no lower outlet, the facility must fill to the level of the top outlet before any water is released, and it does not drain. In addition, a wet basin typically has a standing crop of water-tolerant vegetation along its usual waterline.

A wet basin should have two components: a sediment forebay and a main pool. Runoff first moves through the forebay where gross solids are captured. It then fills the main pool basin until overflowing through an outlet spillway. Properly sized, such a basin will capture the desired volume of water before allowing discharge. In this way it acts as a stilling basin allowing solids to settle. One objective is for the aquatic environment to eliminate pollutants through wetland plant uptake and microbial degradation. In dry climates supplemental water sources may be necessary in order to maintain a pool level supportive of the aquatic environment.

Constructed Wetlands

The concept of a constructed wetland is to gain the pollutant removal characteristics of a natural wetland environment. Among these are settling of solids, wetland plant uptake, and microbial degradation. Extremely wide variations in design are possible. The facility is similar to a wet basin because it must be wet year-round, but it is shallow and marsh-like, creating conditions supporting abundant vegetation and microbial population. Micro-pools, small islands for waterfowl habitat, and multiple species of trees, shrubs, and plants are among the design elements that must be balanced for the facility to be successful.

A constructed wetland has four principal components: a splitter box, a sedimentation forebay, the wetland zone ("pond"), and the outlet structure. The splitter box diverts flow from the main flow path to the entrance, keeping away anything more than the design flow (usually a 25-year storm). From the splitter box, runoff moves into the forebay where gross solids are captured before flowing into the wetland zone. In the wetland zone, runoff moves through multiple irregular flow paths and micro-pool areas filling the wetland "pond" to no more than two feet above its usual water surface elevation. The outlet structure must allow the water level to gradually decrease to its normal elevation. If storm flow rushes through the facility or keeps it inundated too long, the aquatic ecosystem can be damaged. In dry climates supplemental water sources may be necessary in order to maintain a water level supportive of the aquatic environment.

Appendix F
Glossary

APPENDIX F – GLOSSARY

Abbreviated Drainage Plan

A brief written plan stating and schematically showing how a small proposed land development project will satisfy stormwater management requirements of these Guidelines. Generally this is applicable only to projects that will be devoid of detention facilities and public stormwater infrastructure of any kind. This may be accomplished with a site plan showing vertical dimensional controls or a site grading plan.

Above-Project Area

Land area(s) adjoining or near a proposed land development project that contributes stormwater runoff to, or through, the project at the time of hydrologic analysis or in the future. Above-project areas are included in the **drainage study area**.

Anticipated Development

Full potential urbanization of a basin or watershed area in compliance with the Comprehensive Plan. Such an area may include one or more subdivisions, one or multiple property holdings, wholly undeveloped land or both developed and undeveloped land areas.

Area Engineer

The Texas Department of Transportation (TxDOT) operates several Area Offices, each of which has responsibility for several counties. The engineer in charge of each Area Office has the title of Area Engineer.

Areas (Hydrologic)

For uniformity of meaning within these Guidelines land areas are defined according to the general hierarchy listed below. Specific definitions of each are included in the Glossary.

- Watershed (area)
  - Basin (area)
    - Drainage Study Area
      - Project Area
        - Above-Project Area
          - Pathway Area
            - Design Drainage Area

Base Flood

The flood having a one percent chance of being equaled or exceeded in any given year, also known as “100 year” flood.

Basin

A land area making up a portion of a watershed. A basin can be thought of as the entire area contributing storm flow to a watercourse serving as a tributary to a principal named stream. Several basins usually comprise a watershed.

Buildout Condition

Full completion of any land development project in all of its phases, if any, representing the entire contiguously owned tract(s), whether proposed for near-term or possible future development. This refers to: completion of any single-lot site project; the final completion of any multi-stage project entailing a site project staged over time; or final completion of multiple subdivision projects collectively making up a parent tract (or preliminary plat submittal) representing ownership of an un-platted parcel of land regardless of size.

BFE – Base Flood Elevation

The high water surface elevation(s) along a watercourse resulting from the base flood passing down that watercourse.

CFS

A measure of water flow in cubic feet per second

Cibolo Design and Construction Manual Specifications

All items pertinent to design or construction of stormwater facilities of any kind included in the latest adopted version of the Cibolo UDC and Design and Construction Manual, as each may be amended.

City

City of Cibolo

City Engineer

The official city engineer of Cibolo

CLOMR

Conditional Letter of Map Revision as related to FEMA requirements for managing FEMA-designated flood prone areas.

Comprehensive Plan

The urban general plan officially adopted by the City.

Conveyance Pathway

An identifiable route by which concentrated (non-sheet flow) stormwater will travel within and from a project area to a discharge point at a main channel of the Primary Drainage System.

County Engineer

The principal person in Guadalupe County government who has responsibility for engineering decisions.

Conveyance Pathway Area

See "Pathway Area"

Datum Any level surface to which elevations are referred (for example, mean sea level); is also referred to as datum plane, although it is not actually a plane.

Design Drainage Area

The surface area contributing stormwater runoff to any particular point of design in a stormwater management system of any kind. Examples can range in size from the area contributing to a single curb inlet, to that contributing to a flood control facility astride a major stream. Depending on the point of design, the design drainage area can equal an entire watershed, an entire basin, a drainage study area, an off-project area, a project area or portion(s) of any of these areas.

Detention

Temporary storage and metered release of stormwater.

Detention Facility

A permanent facility designed for the temporary storage and metered release of stormwater without creating a permanent pool of water.

Discharge

Stormwater out flow from an area of any kind, or from a storm water feature such as a conduit or a detention facility.

Drainage Development Permit

A permit issued by the City that allows the start of clearing, grubbing, or earthwork as the early stage(s) of a land development project, based on an approved drainage plan or an approved abbreviated drainage plan.

Drainage Easement

An interest in land granted to the City for the maintenance of a drainage facility, on which certain uses are prohibited; and providing for the entry and operation of machinery and vehicles for maintenance purposes.

Drainage Facilities

All elements (public and private) necessary to manage and convey stormwater runoff from its initial contact with earth to its disposition in a watercourse making up the primary drainage system of the Cibolo area. These may include but are not limited to storm sewers, improved channels, unimproved drainage ways, areas within drainage easements or drainage right of way providing concentrated or overland sheet flow, and all appurtenances to the foregoing, such as inlets, manholes, junction boxes, headwalls, culverts, etc.

Drainage Plan

A detailed representation of how stormwater will be managed as part of a proposed land development project (site or subdivision). Usually accompanied by (or incorporated into) an engineering report, it is to be based on an approved preliminary drainage plan.

Drainage Report

A report, prepared by a Registered Professional Engineer, that presents the drainage plan for a land development project (site or subdivision) in compliance with the provisions of these Guidelines. It must document the hydrologic and hydraulic analyses accomplished to address the project area, above-project area(s) and pathway area(s), and any watercourse conveying stormwater to or from the project area.

Drainage Study

See "Drainage Report".

Drainage Study Area

The full extent of land area that must be analyzed for the effects of stormwater runoff, whether part of a project, upland of the project, or contributing stormwater runoff to the conveyance pathway downstream of the project. The drainage study area is equal in size to the sum of the project area, the above-project area, if any, and the pathway area, if any.

Drainage Right Of Way

An area of land dedicated to the City for the purposes of conveying and containing stormwater flow, constructing drainage facilities, and/or allowing entry and/or operation of equipment for maintaining such drainage features and facilities.

Elevation

The vertical distance from a datum, usually the NGVD, to a point or object. For example, if the elevation of point "A" is 802.46 feet, point "A" is 802.46 feet above some datum.

Encroachment

Existing or proposed buildings, foundations, drainage structures, streets (including bridges and culverts), utilities, or earthwork of any kind which is situated in floodplain, or flood fringe areas, the geographic limits of which are defined on the official Flood Insurance Rate Maps of the City.

Equal Encroachment

Equitable encroachment into floodplain or flood fringe areas along a significant reach of both sides of a watercourse, as a function of "low side" and "high side" hydrologically proportionate areas.

Engineer

A Registered Professional Engineer duly authorized and licensed, under provisions of the Texas Engineering Practice Act, to practice the profession of engineering.

Erosion

The process whereby the surface of the earth is loosened and carried away by the action of wind, water, gravity, ice, or a combination thereof.

Existing Condition

The hydrologic condition of the project area or the drainage study area that exists (or existed) prior to any proposed land development work and at the time for which a hydrologic analysis is conducted. Where man-made topographic features predate adoption of these Guidelines, such features shall be considered "exiting condition."

Extraterritorial Jurisdiction (ETJ)

Within the terms of the Texas Municipal Annexation Act, means the unincorporated area, not a part of any other city, which is contiguous to the Corporate Limits of the City, the outer boundaries of which are measured from the extremities of the corporate limits of the City outward for such distances as may be stipulated in the Texas Municipal Annexation Act, in which area, within the terms of the act, the City may enjoin the violation of its subdivision control ordinance.

FEMA Federal Emergency Management Agency of the US Government

F.H.A. Federal Housing Administration, an agency of the US Government.

Flood Insurance Map

See "Flood Insurance Rate Map"

Flood Insurance Rate Map

Any of a series of maps published by FEMA that depicts the geographic limits of flood prone areas along the principle watercourses of the City, for the purpose of identifying those areas in which property owners are eligible to participate in the National Flood Insurance Program.

Floodplain

Overbank areas along a watercourse that are subject to inundation by stormflow due to unusually larger storms events.

Flood Study

The official study, or collection of studies, that defines the flood plains, flood fringe, and floodways of the primary drainage system and tributaries thereof as required in connection with the National Flood Insurance Program sponsored by FEMA.

Floodway

The channel and adjacent overbank areas of a river or other watercourse that may not be filled or hydraulically altered if such fill or alterations will cause a cumulatively increase in the base flood elevation of more than one foot.

Freeboard

That portion of a channel bank, detention embankment, or other stormwater management facility that is above the water surface elevation expected to be generated by the design storm for which the facility is designed.

Guidelines

The design guidelines referenced in this document: "Cibolo Uniform Stormwater Design Guidelines"

Hydraulics

A branch of science that deals with practical applications (such as the transmission of energy or the efforts of flow) of liquid (such as water) in motion.

Hydrology

A science dealing with the properties, distribution, and circulation of water on the surface of the land, in the soil and underlying rocks, and in the atmosphere.

Land Development Project

Any proposed site development or subdivision project requiring building permit(s) or platting under provisions of City ordinances.

Legal Lot

A parcel of land having been divided from a parent tract via a plat duly processed and approved by the City, and filed of record in county records under the platting provisions of Texas State Law.

LOMA Letter of Map Amendment as related to FEMA requirements for managing **FEMA**-designated flood prone areas.

LOMAR

Letter of Map Revision as related to FEMA requirements for managing FEMA-designated flood prone areas.

Lowest Floor

The lowest floor, or the lowest enclosed area (including basement), of a structure. An unfinished or flood resistant enclosure, usable solely for the parking of vehicles, building access or storage, in an area other than a basement area, is not considered a building's lowest floor, provided that such enclosure is not built so as to render the structure in violation of the applicable non-elevation design requirements of City ordinances.

Master Drainage Plan

An official plan of the City for comprehensive management of stormwater runoff in an entire basin or watershed, or in specific reaches thereof.

Mean Sea Level (MSL)

The average height of the surface of the sea for all stages of the tide taken over a 19-year period.

Named Regulatory Watercourse

The major watercourses or streams in the Cibolo region having been ascribed with names and listed in Table B-1, Appendix B.

Natural Land
The cover and topography of land before any man-made changes that would substantively affect the path or intensity of stormwater runoff.

Natural Watercourse
A stream, waterway, or channel more or less in the alignment created by natural forces, with our without man-made alteration of its surfacing and configuration at limited locations.

Pathway Area
Land area(s) that drain to the conveyance pathway of a project, but that are not included in the project area or above-project area(s). See conveyance pathway area.

Principal Named Streams (Watercourses)
See "Named Regulatory Watercourses"

Preliminary Drainage Plan
See "Preliminary Drainage Report"

Preliminary Drainage Report
A report showing a schematic representation of how stormwater will be managed as part of a proposed land development project. It will document pertinent topographic, hydrologic, and land ownership characteristics of all land areas contributing stormflow to a project area, as well as all hydrologic parameters proposed for analysis of design stormflow throughout the project.

Project Area
The entire land area of a proposed site development or subdivision project, at buildout condition, into which buildings, structures, and/or street and utility facilities are to be constructed. This area(s), together with any above-project area(s) and pathway area(s) make up the drainage study area that must be considered in developing plans for stormwater management facilities for the project.

Project Site
See "Project Area"

Reach A length or portion of a watercourse, whether wholly natural or influenced by man-made improvements or alterations.

Regional Detention
A flood control facility approved by the City as a mechanism for managing stormwater runoff form a large land area comprised of one or more subdivisions, one or multiple property holdings, developed and undeveloped land areas, or any combination of such areas.

Regulatory Watercourses
See "Named Regulatory Watercourses"

Regulatory Watershed

The total land area that contributes stormwater runoff to a named regulatory watercourse in the Cibolo region. Each such watercourse has a watershed area that is made up of basins. The sum of the land area(s) in a watershed's basins equals the land area of the watershed.

Retention Facility

A facility that provides for the storage of stormwater flows by means of a permanent pool of water or a permanent pool in conjunction with a temporary storage component.

Right of Way

Land set aside for street and storm drain facilities or utilities, or exclusively for stormwater management purposes.

Sedimentation

Deposits of detached soil particles or rock fragments after being transported from their site or origin by runoff water.

Site See "Site Project".

Site Project

A land area consisting of a single platted lot or two or more contiguous platted lots upon which a building project is planned, consisting of building structures, parking, and other facilities and exclusive of public streets. A site project may or may not include public utilities situated in easements, or stormwater management facilities situated in drainage right of way. See "Site"

Special Design

Any stormwater management facility or technique the design of which is not specifically addressed by these Guidelines or the Cibolo Design and Construction Manual.

Standard Specifications for Construction

See Technical Specifications

Stormwater Planning Conference

A meeting between property owners/developers (including their representatives) and the City Engineer (or his/her designee) for the purpose of identifying how these Guidelines and the provisions of stormwater management ordinances relate to land area(s) proposed for near-term or future development.

Structure

A walled and roofed building that is principally above the ground, as well as a manufactured home.

Study Limits

Associated with a drainage study for a drainage report, this is the geographic limits of the hydrologic and hydraulic analyses that are required for the study.

Subdivision Project

A land development project involving the division of land into lots and ROW for public streets and utilities or the dividing of land into individual lots for near term construction or planned long term construction of site projects.

Surveyor

A Registered Public Surveyor or Registered Land Surveyor as licensed by the State of Texas.

Swale A shallow drainage way characterized as having a "V" shape the sides of which have very flat slopes, generally on the order of sides 6 horizontal to 1 vertical (6:1) or flatter.

Technical Design Summary

A drainage report format that may be used in lieu of a traditional prose report. Following a question/answer process, it is to use the forms provided in Appendix D, with attachments as needed.

Technical Specifications

See "Cibolo Technical Specifications"

Tributaries

Waterways, watercourses, streams, or creeks that directly flow into the Named Regulatory Watercourses of the Cibolo and region. Some may be referred to by a name on maps or other reference.

TxDOT Texas Department of Transportation.

Ultimate Development

This term generally relates to the extent to which impervious materials and plant growth will, at some future time, cover land contributing stormwater runoff to one or more design points in a stormwater management system. Of necessity this requires some plan or a series of assumptions about future characteristics of undeveloped areas. See Anticipated Development

Watercourse

Any depression, channel, storm sewer, or culvert serving to give direction to a current of stormwater.

Watershed

See "Regulatory Watershed"

Appendix G
General References

SECTION IX

APPENDIX G – REFERENCES

The following sources were consulted directly or indirectly by reference in the development of these Guidelines:

Unified Stormwater Guidelines, Bryan-College Station, Texas, 2009

Drainage Criteria Manual, City of Temple, November 1996.

Drainage Criteria Manual, Montgomery County, 1989.

Drainage Manual, City of Austin, June 1993.

Bryan College Station Unified Stormwater Guidelines 2009.

Environmental & Municipal Update, Lloyd Gosselink, Attorneys at Law, April 2005

Environmental & Municipal Update, Lloyd Gosselink, Attorneys at Law, January 2006.

Erosion and Sediment Control Guidelines for Developing Areas in Texas, Soil Conservation Service, US Department of Agriculture.

"Erosion and Sedimentation Control Measures", short course by Engineering Utilities and Public Works Training Institute, Texas Engineering Extension Service, Texas A&M University System, 2003.

Haestad Method's Culvert Master

Hydraulic Design Manual, Texas Department of Transportation, November 2002.

Mitigation Guidelines Regulatory Program, Fort Worth District, US Army Corps of Engineers, December 2003.

National Menu of Best Management Practices For Stormwater Phase II, US Environmental Protection Agency, August 2002.

Regulatory Program Overview, Fort Worth District, US Army Corps of Engineers, March 2003

Rossmiller, R.,L. "The Rational Formula Revisited"

Urban Hydrology for Small Watersheds, Technical Release No. 55, Soil Conservation Service (National Resource Conservation Service) US Department of Agriculture, June 1986.

Walsh (1989) from Chow (1959)

Water Resources and Environmental Engineering